WGCC - IMAGE BUILDERS

WINTER GARDEN COMMERCE CENTER

690 GARDEN COMMERCE PARKWAY WINTER GARDEN, FLORIDA 34787

OWNER:

LBI PROPERTIES, LLC

WINDERMERE, FLORIDA 34786-2094 (321) 229-1284

CONTACT: TOM HARVEY EMAIL: TOM@IMAGEHOMEBUILDERS.COM

CIVIL ENGINEER:

UNROE ENGINEERING, INC.

ORLANDO, FLORIDA 32869

CONTACT: DARCY UNROE, PE **EMAIL: DARCY@UNROEENGINEERING.COM**

ARCHITECTURE, MECHANICAL, PLUMBING AND ELECTRICAL:



C4 ARCHITECTURE, LLC

ORLANDO, FLORIDA 32801 PHONE: (407) 363-6136

ARCHITECTURE CONTACT: CORY COOPER EMAIL: CCOOPER@C4ARCHITECTURE.COM

STRUCTURAL CONTACT: JONATHAN D. COLLINS, PE, SI EMAIL: JCOLLINS@C4ARCHITECTURE.COM

MECHANICAL & PLUMBING CONTACT: JOSE ZAMOT, PE EMAIL: JZAMOT@C4ARCHITECTURE.COM **ELECTRICAL CONTACT: DREW LILES, PE**

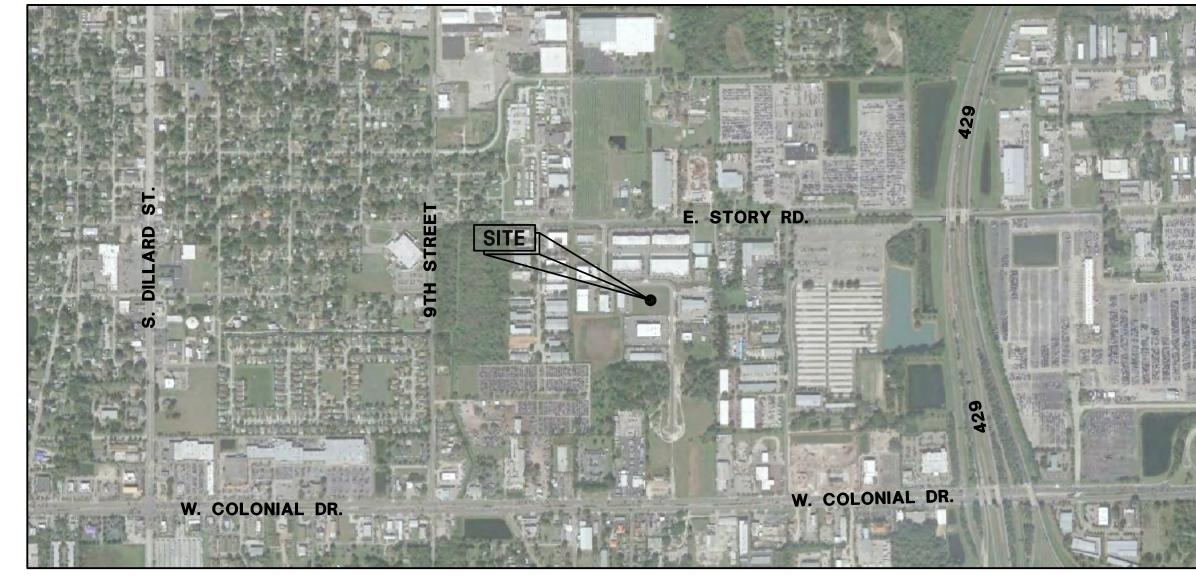
FIRE PROTECTION ENGINEER:

WIGINTON FIRE PROTECTION ENGINEERING, INC.

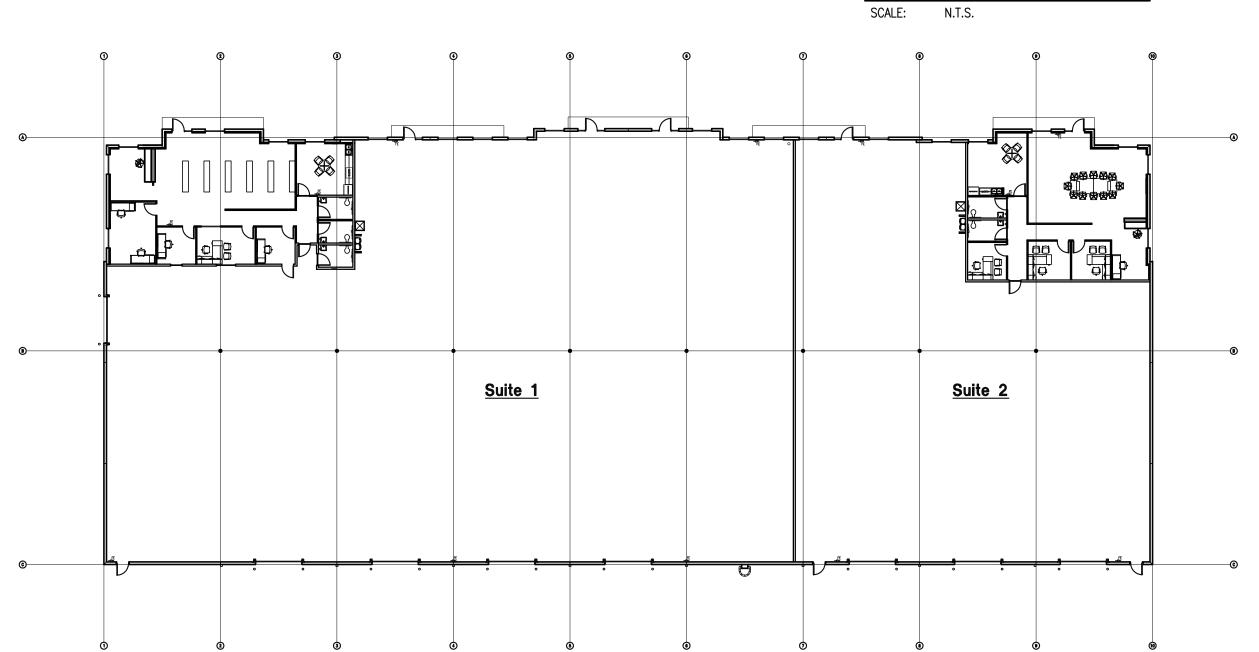
1501 NORTHPOINT PKWY STE 1012 WEST PALM BEACH, FLORIDA 33407 PHONE: (561) 324-6731

EMAIL: DLILES@C4ARCHITECTURE.COM

CONTACT: RANDY ALMOND RDA@WIGINTON.NET



VICINITY MAP



KEY PLAN HARBCO GENERAL CONTRACTORS, INC.

CODE INFORMATION

PROJECT SQUARE FOOTAGE:

STORAGE (S-1) = TOTAL BUILDING S.F =	25,933 SQ. FT 29,794 SQ. FT
CONSTRUCTION TYPE:	

TYPE II-B - UNPROTECTED, SPRINKLERED (FBC)

OCCUPANCY: S-1, B, M - BUSINESS PER FLORIDA BUILDING CODE, SEVENTH EDITION (2020) NO SEPARATION REQUIRED PER SECTION 508.3.3. AND

APPLICABLE BUILDING CODES:

TABLE 508.4 OCCUPANCY SEPARATION.

- FLORIDA BUILDING CODE, SEVENTH EDITION (2020)
- FLORIDA ENERGY CODE, SEVENTH EDITION (2020) 2016 ASHRAE 90.1
- FLORIDA MECHANICAL CODE, SEVENTH EDITION (2020) • FLORIDA PLUMBING CODE, SEVENTH EDITION (2020)
- EXISTING BLDG. FLORIDA BUILDING CODE, SEVENTH EDITION (2020)
- FLORIDA ACCESSIBILITY CODE, SEVENTH EDITION (2020) • FLORIDA FIRE PREVENTION CODE, SEVENTH EDITION (2020)
- 2017 NATIONAL ELECTRIC CODE
- NFPA 101 LIFE SAFETY CODE, 2018 EDITION NFPA 10 STANDARD FOR PORTABLE FIRE EXTINGUISHERS, 2018 EDITION

PLUMBING FIXTURES

				LORIDA BUILDING COL	DE — 7TH EDITI
	CLOSETS	=	ORIES	DRINKING	OTHER
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	
1 PER 50 E	OR FIRST 50/ XCEEDING 50 .56 FIXTURES	1 PER 80 E	OR FIRST 80/ XCEEDING 80 .35 FIXTURES	1 PER 100	_
.28 MIN.	.28 MIN.	.175 MIN.	.175 MIN.	.14 MIN.	
REQUIRED FIXT	URES - ASSEM	BLY (A-2 = 1	7 OCCUPANTS)	(A-3 = 24 OCCUPA)	NTS)
WATER (CLOSETS	LAVAT	ORIES	DRINKING	OTUED
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	OTHER
1 PE 17 PEOPLE	R 75 = .23 FIXTS.		R 200 = .085 FIXTS.	1 PER 500	1 SERVICE SINK
.11 MIN.	.11 MIN.	.425 MIN.	.425 MIN.	.034 MIN.	SINK
1 PER 125 24 PEOPLE = .192 FIXTS.	1 PER 65 24 PEOPLE = .36 FIXTS.	1 PER 200 24 PEOPLE = .12 FIXTS.		1 PER 500	1 SERVICE
.192 MIN.	.36 MIN.	.06 MIN.	.06 MIN.	.048 MIN.	SINK
REQUIRED FIXT	JRES – MERCA	NTILE 13 OCCU	JPANTS		•
WATER (CLOSETS	LAVAT	ORIES	DRINKING	OTLIED
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	OTHER
1 PER 500 13 PEOPLE = .026 FIXTURES		1 PEF 13 PEOPLE =	R 750 .017 FIXTURES	1 PER 1000	-
.013 MIN.	.013 MIN.	.008 MIN.	.008 MIN.	.013 MIN.	
REQUIRED FIXT	URES - STORA	GE 53 OCCUPA	NTS		
WATER (CLOSETS	LAVATORIES		DRINKING	OTHER
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	OTHER
	R 100 = .53 FIXTS.	1 PEF 53 PEOPLE	R 100 = .53 FIXTS.	1 PER 1000	1 SERVICE
.265 MIN.	.265 MIN.	.265 MIN.	.265 MIN.	.053 MIN.	SINK
TOTAL REQUIRE	D FIYTHRES.				
· · · · · · · · · · · · · · · · · · ·	OLOCETC	LAVAT			

WATER	CLOSETS	LAVAT	ORIES	DRINKING	OTHER
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	OTHER
1 MIN.	2 MIN.	1 MIN.	1 MIN.	2 MIN.	2 SERVICE SINK
TOTAL PROVIDE	D FIXTURES:				
WATER	CLOSETS	LAVAT	ORIES	DRINKING	OTHER
MALE	FEMALE	MALE	FEMALE	FOUNTAINS	OTHER
2	3	2	3	4 MIN.	2 SERVICE SINK
			-		

LIFE SAFETY SUMMARY

OCCUPANT LOAD			(PER FLORIDA BUILDING CODE - 7TH EDITION
SUITE 1:			
BUSINESS (B OCCUPANCY):			
1,141 S.F. (BUSINESS USE)	=	8	OCCUPANTS (1 PER 150 S.F.)
ASSEMBLY (A-2 OCCUPANCY):			
219 S.F. (BUSINESS USE)	=	8	OCCUPANTS (1 PER 30 S.F.)
MERCANTILE (M OCCUPANCY):			
728 S.F. (BUSINESS USE)	=	13	OCCUPANTS (1 PER 60 S.F.)
STORAGE (S-1 OCCUPANCY):			
17,565 S.F. (STORAGE USE)	=	36	OCCUPANTS (1 PER 500 S.F.)
TOTAL SUITE OCCUPANTS =	65		
SUITE 2: BUSINESS (B OCCUPANCY):			
810 S.F. (BUSINESS USE)	=	6	OCCUPANTS (1 PER 150 S.F.)
ASSEMBLY (A-2 OCCUPANCY):			
,	=	9	OCCUPANTS (1 PER 30 S.F.)
ASSEMBLY (A-3 OCCUPANCY):		0.4	0001/01/170 /4 DED 70 05
· · · · · · · · · · · · · · · · · · ·	=	24	OCCUPANTS (1 PER 30 S.F.)
STORAGE (S-1 OCCUPANCY):		47	00011041170 (4, DED, 500, 0.5)
		17	OCCUPANTS (1 PER 500 S.F.)
TOTAL SUITE OCCUPANTS = TOTAL OCCUPANTS = 121	56		

MEANS OF EGRESS

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MICANS OF EGRESS
BUSINESS (B USE):
MAXIMUM ALLOWABLE TRAVEL DISTANCE = 300' FT. (FULLY SPRINKLED) MAXIMUM PROVIDED TRAVEL DISTANCE = 105'-1" FT. MAXIMUM ALLOWABLE DEAD END CORRIDOR LENGTH = 50' FT. MINIMUM ALLOWABLE AISLE WIDTH = 44 IN.
EGRESS WIDTH PER PERSON SERVED = .2 IN2 INCHES x 68 OCCUPANTS = 13.6 IN. REQUIRED (xx IN. PROVIDED)
MEANS OF EGRESS

STORAGE (S-1 USE):	
MAXIMUM ALLOWABLE TRAVEL DISTANCE MAXIMUM PROVIDED TRAVEL DISTANCE MAXIMUM ALLOWABLE COMMON TRAVEL PATH MAXIMUM ALLOWABLE DEAD END CORRIDOR LEY	= 400' FT. (FULLY SPRINKLE) = 125'-3" FT. = 100' FT. (FULLY SPRINKLE)
MINIMUM ALLOWABLE AISLE WIDTH	= 44 IN.
EGRESS WIDTH PER PERSON SERVED .2 INCHES x 53 OCCUPANTS = 10.6 I	= .2 IN. IN. REQUIRED (xx IN. PROVIDED)

SHEET INDEX

AR	CHITEC	TUR	AL
•	12-15-22] A000	COVER SHEET
9	12-15-22	A001	OVERALL SITE PLAN
•	12-15-22	A100	LIFE SAFETY PLAN
•	12-15-22	A200	OVERALL FLOOR PLAN
•	12-15-22	A201	ENLARGED FLOOR PLANS
9	12-15-22	A202	REFLECTED CEILING PLAN
•	12-15-22	A203	ENLARGED RESTROOM PLANS
•	12-15-22	A204	ENLARGED BREAK ROOM PLAN & MILLWORK
•	12-15-22	A300	EXTERIOR ELEVATIONS
•	12-15-22	A301	ROOF PLAN
•	12-15-22	A400	EXTERIOR WALL SECTIONS
•	12-15-22	A401	EXTERIOR WALL SECTIONS
•	12-15-22	A402	EXTERIOR WALL SECTIONS
•	12-15-22	A403	INTERIOR WALL SECTIONS
•	12-15-22	A500	SCHEDULE & DETAILS
•	12-15-22	A801	DUMPSTER SHEET

SI	RU	JCT	'UR

		,	
•	12-15-22	S001	GENERAL STRUCTURAL NOTES
•	12-15-22	S002	DESIGN LOADS
•	12-15-22	S101	FOUNDATION PLAN
•	12-15-22	S111	ROOF FRAMING PLAN
•	12-15-22	S401	TYPICAL TILT-UP WALL DETAILS
•	12-15-22	S402	TILT-UP PANEL ELEVATIONS
•	12-15-22	S501	SCHEDULES
•	12-15-22	S502	SCHEDULES
•	12-15-22	S601	FOUNDATION SECTIONS & DETAILS
•	12-15-22	S602	FOUNDATION SECTIONS & DETAILS
•	12-15-22	S603	DUMPSTER PAD PLAN & DETAILS
•	12-15-22	S611	FRAMING SECTIONS & DETAILS
•	12-15-22	S612	FRAMING SECTIONS & DETAILS
•	12-15-22	S613	FRAMING SECTIONS & DETAILS

MECHANICAL

e e e e	12-15-22	M001	MECHANICAL GENERAL NOTES
	12-15-22	M100	MECHANICAL OVERALL PLAN
	12-15-22	M300	MECHANICAL PLANS SUITE 1
	12-15-22	M301	MECHANICAL PLANS SUITE 2
•	12-15-22	M301	MECHANICAL PLANS SUITE 2
	12-15-22	M600	MECHANICAL DETAILS
	12-15-22	M700	MECHANICAL SCHEDULES

PLUMBING

	ECTRIC		I LOMIDING MIJENJ
•	12-15-22 12-15-22	P300 P600	PLUMBING PLANS, DETAILS & SCHEDULES PLUMBING RISERS
	12-15-22	P100	PLUMBING OVERALL PLAN
•	12-15-22	P001	PLUMBING GENERAL NOTES

•	12-15-22	E100	ELECTRICAL NOTES, SPECIFICATIONS, SYMBOLS & ABBREVIATIONS
•	12-15-22	E101	POWER RISER DIAGRAM, NOTES & SPECIFICATIONS
•	12-15-22	E102	ELECTRICAL SCHEDULES
•	12-15-22	E200	ELECTRICAL SITE PLAN
•	12-15-22	E201	SITE PHOTOMETRICS PLAN
•	12-15-22	E202	SITE PHOTOMETRICS CUT SHEETS
•	12-15-22	E203	SITE PHOTOMETRICS CUT SHEETS
•	12-15-22	E204	SITE PHOTOMETRICS CUT SHEETS
•	12-15-22	E300	ELECTRICAL PLAN — AREA 'A'
•	12-15-22	E301	ELECTRICAL PLAN - AREA 'B'
•	12-15-22	E302	ELECTRICAL PLAN - AREA 'B'
•	12-15-22	E400	RISERS & DETAILS
•	12-15-22	E401	UL DETAILS

NOTES:

ROOF STRUCTURE IS <u>NOT</u> LIGHT GAUGE METAL FRAMING.
IT IS THE CONTRACTOR'S RESPONSIBILITY TO FURNISH AND INSTALL ALL NECESSARY COMPONENTS FOR THE PROPER OPERATION OF ALL SPECIFIED SYSTEMS, REGARDLESS OF WHETHER THOSE COMPONENTS ARE SHOWN ON THE DRAWINGS OR NOT. CONTRACTOR SHALL ALSO VERIFY THAT SPECIFIED COMPONENTS ARE COMPATIBLE WITH OTHERS WITHIN THE SAME SYSTEM AND WITH THE SYSTEM ITSELF.
"SEPARATE PERMITS REQUIRED FOR: DUMPSTER ENCLOSURE, SIGNS, SPRINKLERS, FIRE ALARM, AND BURGLAR ALARM."
"STRUCTURAL STEEL WORK SHALL BE INSPECTED AND CERTIFIED BY AN APPROVED 3RD PARTY PRIOR TO FRAMING INSPECTION."
STRUCTURAL STEEL WORK CERTIFICATION, FROM 3RD PARTY INSPECTOR, DOCUMENTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO CO APPLICATION.

SUBMIT APPROVED SHOP DRAWINGS FOR ROOF JOISTS AND TILT-UP WALL CONSTRUCTION TO BUILDING DEPARTMENT PRIOR TO INSPECTION. (DETAILING FOR JOIST PLACEMENT AND SIZES ARE ON SUBMITTED PLANS AS ARE DETAILS OF TILT-UP WALLS).

MUNICIPALITY: CITY OF WINTER GARDEN

PARCEL ID #: 24-22-27-9385-00-130

PROPERTY DESCRIPTION:

WINTER GARDEN COMMERCE CENTER PHASE 1 72/133 LOT 13

PROJECT NO.: 18406

DATE: SEPTEMBER 16, 2022



STRUCTURAL ABBREVIATIONS ANCHOR BOLT LENGTH ABOVE LONG AMERICAN CONCRETE INSTITUTE LIVE LOAD LONG LEG HORIZONTAL ADDITIONAL ABOVE FINISH FLOOR LONG LEG VERTICAL A.I.S.C. AMERICAN INSTITUTE OF STEEL CONSTRUCTION LONG LONGITUDINAL A.I.S.I. AMERICAN IRON AND STEEL INSTITUTE LOW POINT ALTERNATE ARCH ARCHITECT / ARCHITECTRUAL A.S.T.M. AMERICAN SOCIETY OF TESTING MATERIALS MASONRY MAX A.W.S. AMERICAN WELDING SOCIETY MAXIMUM MASONRY BEAM METAL BUILDING MANUFACTURER **BOTTOM OF** MOMENT CONNECTION BLDG. BUILDING MASONRY CONTROL JOINT BLW BELOW MECH **MECHANICA** MEZZ MEZZANINE OR MECH PLATFORM BEAM BOTTOM MFR MANUFACTURE OR MANUFACTURER BASE PLATE MIN MINIMUM MOM BRDG BRIDGING MOMENT BRG BEARING M.O. MASONRY OPENING BRICK METAL **BOTH SIDES** BTWN BETWEEN **NEAR SIDE** NOT TO SCALE NTS CENTER TO CENTER CONCRETE BEAM **OVFRALI** CANTILEVER CONCRETE COLUMN **OVER BUILT** ON CENTER CAST IN PLACE CONTROL JOIN OUTSIDE DIAMETER C OR CL CENTERLINE OUTSIDE FACE OPNG **OPENING** C.M.U. CONCRETE MASONRY UNIT **OPPOSITE** COL COLUMN COORD CONC COORDINATE CONCRETE P.A.F. POWER ACTUATED FASTENER CONN CONNECTION PRECAST CONT CONTINUOUS PERP **PERPENDICULAR** CONTR CONTRACTOR PRE-ENGINEERED CONST P OR PL CONSTRUCTION PI ATF CONSTRUCTION JOIN PLY **PLYWOOI** P.L.F. POUNDS PER LINEAR FOOT CTR'D CENTERED PREMANUF PRE-MANUFACTURED PRE-FABRICATED P.S.F. POUNDS PER SQUARE FOOT P.S.I. DEAD LOAD POUNDS PER SQUARE INCH DIAMETER PTN PARTITION DIAG DIAGONAL PRESSURE TREATED DIMENSION DISTANCE DOWN DETAIL REFERENCE OR REFER DWG(S) DRAWING(S) REINF REINFORCE(D) OR REINFORCING REQ REQUIRE REQ'D REQUIRED EACH **REVIEWED** EACH END ROOF FACH FACE RTN RETURN **EXPANSION JOINT** RETAINING WALL ENGINEER ELEVATION SCHEDULE EQUAL SCH EQ. SP. EQUAL SPACING SECTION EACH SIDE STEEL DECK INSTITUTE FACH WAY STEP FOOTING **EXTERIOR** SIMILAR SAWCUT JOINT FACE OF STEEL JOIST INSTITUTE FLOOR DRAI FINISH FLOOR SPACE OR SPACES **FOUNDATION** SPECIFICATIONS FINISH FLOOR STAINLESS STEE FAR SIDE STANDARD FOOT STEEL FTG FOOTING STRENGTH S.W. SHEAR WALL SYMM SYMMETRICAL GAGE OR GAUGE GALVANIZED TIE BEAM GRADE BEAM GENERAL CONTRACTOR TOP & BOTTON TEMP **TEMPERATUR** THREADED HOOK THICK HORIZ THICKENED SLAB HORIZONTAL THNS **HIGH POINT** TOP'G TOPPING **HEADED STUD ANCHOR TYPICAL** TOP OF SLAB HEIGH1 TRANS **TRANSVERSE** INSIDE DIAMETER INSIDE FACE **UNLESS NOTED OTHERWISE** INTERIOR VERT VERTICAL JOINT WALL FOOTING OR CONT FOOTING KIP (1000 LBS) WINDOW OPENING KNOCK OUT WORK POINT W.P. KEYWAY WFIGHT W.W.F. WELDED WIRE FABRIC (MESH) WITH

GOVERNING CODES AND STANDARDS

THE STRUCTURAL DESIGN AND ALL WORK REFERENCED HEREIN SHALL CONFORM TO THE FOLLOWING CODES AND STANDARDS. USE THE LATEST EDITION UNLESS NOTED OTHERWISE.

- . "FLORIDA BUILDING CODE" FBC 2020, 7TH EDITION.
- "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" ASCE 7-16.
- B. "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" ACI 318-14.
- 4. "ACI MANUAL OF CONCRETE PRACTICE" PARTS 1 THROUGH 5 LATEST EDITION. "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" - AISC 360-16.
- "STRUCTURAL WELDING CODE STEEL" AWS D1.1-2020.

GENERAL CONDITIONS

- THE GENERAL CONTRACTOR SHALL REVIEW AND VERIFY THAT ALL DIMENSIONS ARE COORDINATED BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO FABRICATION OR START OF CONSTRUCTION.
- THE GENERAL CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL CONDITIONS AT THE PROJECT SITE AND SHALL NOTIFY ARCHITECT/ENGINEER OF DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH ANY WORK.
- THESE STRUCTURAL DRAWINGS ARE TO BE USED IN COMBINATION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DRAWINGS, AND ANY OTHER PROJECT CONTRACT DOCUMENTS NOT LISTED. REFER TO THESE DRAWINGS FOR DETAILS AND INFORMATION THAT MAY RELATE TO STRUCTURAL COMPONENTS.
- THESE STRUCTURAL DRAWINGS AND RELATED SPECIFICATIONS, IF PROVIDED, REPRESENT THE COMPLETED DESIGN OF THE STRUCTURE. THEY DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURE AND SEQUENCE TO ENSURE THE SAFETY OF THE CONSTRUCTION SITE
- OBSERVATION VISITE TO THE SITE BY THE EOR OR REPRESENTATIVES OF THE EOR MAY BE MADE DURING CONSTRUCTION, ANY SUPPORT SERVICES PERFORMED BY THE EOR SHALL BE DISTINGUISHED FROM INSPECTION AND/OR TESTING SERVICES PERFORMED BY OTHERS AND ARE NOT TO BE CONSTRUED AS SUPERVISION AND/OR MANAGEMENT OF CONSTRUCTION.
- THE OWNER WILL ENGAGE A QUALIFIED, APPROVED TESTING AGENCY TO PROVIDE SERVICES AS INDICATED BELOW SUBMIT REPORTS TO STRUCTURAL ENGINEER AND ARCHITECT.
- A. TEST SOIL COMPACTION PER LATEST GEOTECHNICAL REPORT (U.N.O.)
- B. TEST CONCRETE IN ACCORDANCE WITH ASTM C172 AND C31. VISUALLY INSPECT FIELD WELDS, BOLTED CONNECTIONS, AND OTHER STRUCTURAL STEEL CONNECTIONS. ALL FIELD WELDS SHALL BE INSPECTED BY A CERTIFIED WELD INSPECTOR.
- SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THESE STRUCTURAL DRAWINGS OR CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO STRUCTURAL MEMBERS WILL NOT BE PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER.
- IN THE CASE OF CONFLICT BETWEEN THE GENERAL NOTES, DRAWINGS, GOVERNING BUILDING CODES, AND/OR SPECIFICATIONS, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN.

SHOP DRAWINGS AND SUBMITTALS

- THE GENERAL CONTRACTOR SHALL FOLLOW THE ARCHITECT'S INSTRUCTIONS FOR DISTRIBUTION OF SHOP DRAWINGS. 2. SHOP DRAWING REVIEW IS FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT. CORRECTIONS OR COMMENTS MADE ON THIS REVIEW DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY FOR ERRORS AND/OR OMISSIONS. NOR
- 3. APPROVAL OF SHOP DRAWINGS DOES NOT INDICATE AN ACCEPTANCE OF DEVIATIONS FROM THE CONTRACT
- DOCUMENTS OR PREVIOUS SHOP DRAWING REVIEW, UNLESS SPECIFICALLY NOTED THEREIN BY ENGINEER OF RECORD. ANY PROPOSED CHANGE TO THE DESIGN CONCEPTS SHOWN IN THE CONTRACT DOCUMENTS SHALL BE SUBMITTED IN WRITING AND APPROVED BY THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO SUBMITTING SHOP DRAWINGS. ALL SUCH CHANGES SHALL BE "CLOUDED" ON THE SHOP DRAWINGS AND REFERENCED TO THE PROPER R.F.I. NUMBER.
- DETAILER SHALL BE RESPONSIBLE FOR CHECKING ALL ARCHITECTURAL AND MECHANICAL DRAWINGS FOR OPENINGS AND EMBEDS AFFECTING STRUCTURAL MEMBERS.
- 6. SHOP DRAWINGS SHALL BEAR THE INITIALS OF THE DETAILER'S CHECKER AND SHALL BE REVIEWED AND APPROVED BU THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL TO ARCHITECT AND ENGINEER OF RECORD. 7. THE USE OF REPRODUCTIONS OF THESE CONTRACT DRAWINGS, IN WHOLE OR IN PART, BY ANY CONTRACTOR,
- SUBCONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SHALL SIGNIFY HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT, AND OBLIGATED HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.
- 8. IF REPRODUCTIONS OF THESE CONTRACT DRAWINGS ARE USED IN LIEU OF PREPARATION OF SHOP DRAWINGS, THE ARCHITECT'S, ENGINEER'S OR OTHER DESIGN CONSULTANT'S TITLE BLOCK SHALL BE REMOVED AND REPLACED WITH A TITLE BLOCK LISTING THE FOLLOWING ITEMS.
- A. NAME, ADDRESS, AND CONTACT NUMBER OF CONTRACTOR, SUBCONTRACTOR, ETC. SUBMITTING SHOP DRAWINGS B. SHEET NUMBER
- C. DATE DRAWING WAS PREPARED, THE INITIALS OF THE PERSON WHO PREPARED THE DRAWINGS, AND THE INITIALS OF THE PERSON WHO CHECKED THE DRAWINGS.
- 9. ANY REPRODUCTION OF THESE CONTRACT DRAWINGS NOT COMPLYING WITH THE ABOVE WILL BE REJECTED
- 10. SOME STRUCTURAL SYSTEMS INCLUDED IN THESE CONTRACT DRAWINGS ARE INDICATED AS "DESIGNED BY SPECIALTY OR "DELEGATED ENGINEER." DELEGATED ENGINEERING SUBMITTALS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.
- 11. CALCULATIONS AS REQUIRED BY THE DRAWINGS AND/OR SPECIFICATIONS SHALL BE SUBMITTED WITH THE REQUIRED SHOP DRAWINGS. ALL DELEGATED ENGINEERING SUBMITTALS SHALL REQUIRE CALCULATIONS SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.
- 12. SHOP DRAWING AND DELEGATED ENGINEERING SUBMITTAL REQUIREMENTS
- A. INFORMATION SUBMITTALS a. CONCRETE MIX DESIGNS & MISC. PRODUCT DATA

FROM COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.

- b. MASONRY UNITS & GROUT MIX DESIGN c. TILT-UP WALL PRODUCT DATA d. EPOXY AND MECHANICAL ANCHORS
- e. MISCELLANEOUS
- B. SHOP DRAWING SUBMITTALS a. RFINFORCING STFFI
- b. STRUCTURAL STEEL, MISC STEEL EMBEDS/CONNECTIONS c. OPEN-WEB STEEL JOISTS & METAL DECKING
- d. TILT-UP WALL PANELS e. MISCELLANEOUS
- C. DELEGATED ENGINEERING SUBMITTALS a. PRE-FABRICATED/PRE-ENGINEERED CANOPIES, AWNINGS, ETC. b. TILT-UP WALL PANEL LIFTING & BRACING DRAWINGS
- c. MISCELLANEOUS

POST-INSTALLED ANCHORS NOTES

- POST-INSTALLED ANCHOR SYSTEMS SHALL COMPLY WITH THE LATEST REVISION OF ICC-ES ACCEPTANCE CRITERIA AC308 AND HAVE A VALID ICC-ES REPORT IN ACCORDANCE WITH ALL APPLICABLE CODES.
- 2. POST-INSTALLED ANCHOR SYSTEMS MUST BE INSTALLED IN STRICT ACCORDANCE WITH ALL WRITTEN MANUFACTURER INSTRUCTIONS INCLUDING ANY SPECIAL EQUIPMENT REQUIRED.
- 3. THE PRODUCTS LISTED BELOW ARE THE BASIS OF DESIGN FOR THIS PROJECT. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED BELOW SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO INSTALLATION. SUBSTITUTIONS WILL ONLY BE CONSIDERED FOR PRODUCTS THAT HAVE A CODE REPORT RECOGNIZING THE PRODUCT FOR THE APPROPRIATE APPLICATION AND PROJECT BUILDING CODE. SUBSTITUTION SUBMITTALS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE EQUIVALENT PERFORMANCE VALUES OF THE DESIGN BASIS PRODUCT.

4. BASIS OF DESIGN FOR POST-INSTALLED ANCHORS:

- A. ADHESIVE ANCHORS (EPOXY ANCHORS)
- a. INTO CONCRETE: ADHESIVE FOR REBAR AND ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.4 ABD ICC-ES AC308 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. ADHESIVE ANCHORS SHALL BE INSTALLED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER WHERE DESIGNATED ON THE CONTRACT DOCUMENTS. PRE-APPROVED PRODUCTS INCLUDE: HILTI HIT-HY 200
- SIMPSON STRONG-TIE SET-XP SIMPSON STRONG-TIE AT-XP

REGARDLESS OF WHICH EPOXY PRODUCT OR MANUFACTURER IS USED.

- ANCHORING SYSTEMS SHALL UTILIZE TRADITIONAL PREPARATION OF THE ANCHOR HOLE (BLOWING OR BRUSHING) PER THE MANUFACTURER'S WRITTEN REQUIREMENTS. OTHER METHODS SHALL NOT BE USED WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER OF RECORD.
- ANCHORING ADHESIVE SHALL BE A TWO-PART COMPONENT 100% SOLID EPOXY BASED SYSTEM SUPPLIED THROUGH A STATIC-MIXING NOZZLE SUPPLIED BY THE MANUFACTURER. THIS REQUIREMENT SHALL BE MET
- d. THREADED RODS TO BE USED IN COMBINATION WITH EPOXY SYSTEM SHALL BE FABRICATED FROM STEEL MEETING OR EXCEEDING THE PROPERTIES OF ASTM A36.
- B. MECHANICAL ANCHORS (EXPANSION / WEDGE / SCREW ANCHORS):
- INTO CONCRETE: ANCHORS SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED CONCRETE AND SEISMIC APPLICATIONS. PRE-APPROVED MECHANICAL ANCHOR PRODUCTS INCLUDE: HILTI KIWK HUS-EZ (SCREW ANCHOR) HILTI KWIK-BOLT TZ OR KWIK BOLT TZ 2 (EXPANSION ANCHORS)
- SIMPSON STRONG-TIE TITEN-HD (SCREW ANCHOR) SIMPSON STRONG-TIE STRONG-BOLT 2 OR WEDGE-ALL (EXPANSION ANCHORS)

FOUNDATION AND GEOTECHNICAL NOTES

- NO GEOTECHNICAL REPORT WAS PROVIDED FOR THIS SITE AT THE TIME OF THESE CONTRACT DRAWINGS. THE OWNER SHALL ENGAGE A GEOTECHNICAL ENGINEER TO PROVIDE AN INDUSTRY STANDARD GEOTECHNICAL ENGINEERING SERVICES REPORT FOR THE SITE CONDITIONS AND THE PROPOSED STRUCTURE. SUBMIT COMPLETED REPORT TO THE ARCHITECT AND STRUCTURAL ENGINEER OF RECORD PRIOR TO COMMENCEMENT OF ANY WORK.
- THE ASSUMED ALLOWABLE SOIL BEARING PRESSURE IS 2000 PSF. THIS ASSUMED SOIL BEARING PRESSURE IS BASED ON TABLE 1806.2 OF THE FLORIDA BUILDING CODE - **SANDY SOILS**. UPON REVIEW OF THE CONPLETED GEOTECHNICAL ENGINEERING REPORT, THIS ASSUMPTION WILL BE REVIEWED. REVISIONS TO ANY FOUNDATIONS AS REQUIRED WILL BE COMPLETED. ANY CHANGES TO THE DESIGN INTENT SHALL BE THE RESPONSIBILITY OF THE BUILDING OWNER.
- ALL REQUIREMENTS FOR SITE PREPARATION AND SOIL COMPACTION SPECIFIED IN THE GEOTECHNICAL REPORT SHALL BE FOLLOWED UNLESS ADDITIONAL MORE STRINGENT REQUIREMENTS ARE SPECIFIED. A CERTIFIED TESTING AGENCY SHALL PERFORM SOIL DENSITY AND COMPACTION TESTS TO ENSURE CONFORMANCE WITH THE GEOTECHNICAL REPORT.
- SUBMIT ALL TESTS RESULTS TO THE PROJECT ARCHITECT AND ENGINEER. TEST PER THE FOLLOWING: PAVED AND BUILDING SLAB AREAS: AT SUBGRADE AND AT EACH COMPACTED FILL LAYER, AT LEAST ONE TEST FOR EVERY 2000 SQ. FT., BUT IN NO CASE FEWER THAN 3 TESTS.
- FOOTINGS: AT EACH COMPACTED BACKFILL LAYER AT EACH FOOTING OR ONE TEST FOR EACH 50 FT. OF WALL CONTRACTOR SHALL RECOMPACT AND RETEST UNTIL SPECIFIED COMPACTION IS OBTAINED.
- CONTRACTOR, IN CONJUNCTION WITH THE PROJECT GEOTECHNICAL ENGINEER, SHALL VERIFY EXISTING FIELD
- CONDITIONS DURING EXCAVATION THAT MAY AFFECT THE ALLOWABLE BEARING PRESSURE AND / OR THE INSTALLATION OF THE FOUNDATION SYSTEM PRIOR TO STARTING WORK. ALL FOOTINGS SHALL BE CENTERED UNDER THE COLUMN OR WALL ABOVE UNLESS NOTED OTHERWISE.
- CONCRETE FOR THE FOUNDATIONS SHALL BE PLACED WITHIN 24 HOURS OF THE SUB-GRADE APPROVAL BY THE PROJECT
- GEOTECHNICAL ENGINEER OR THEIR REPRESENTATIVE.
- TERMITE TREATMENT REQUIREMENT BY FBC 109.4 FOR ALL CONSTRUCTION B. FOR EXTERIOR CONCRETE AGAINST THE BUILDING: THE SOILS MUST BE TREATED FOR SUBTERRANEAN TERMITES PER FBC 1816.1.6 AND PROTECTED WITH A VAPOR RETARDER PER FBC 1816.1.4.
- TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMICIEDS, INCLUDING SOIL APPLIED PESTICIDES,

 BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELLED FOR USE AS A PREVENTATIVE TREATMENT TO NEW CONSTRUCTION. PER FBC FOR REGISTERED TERMICIDE. UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PET CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

CAST-IN-PLACE CONCRETE NOTES

- 1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE PUBLICATIONS "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE," - ACI 318 LATEST EDITION, AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS," - ACI 301 LATEST EDITION.
- 2. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS IN ACCORDANCE WITH THE FOLLOWING "SCHEDULE OF CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS":

COLIED II E OF CAST IN DIACE CONCEPTE CONCEDITON MATERIALS

SCHE	DULE OF CAST-IN-PLA	ACE CONCRETE CONSTRUCTI	<u>ON MATERIALS</u>
	LOCATION	28-DAY COMPRESSIVE STRENGTH, fc	MAX w/c RATIO
	FOOTINGS, FOUNDATIONS	4,000 PSI	0.52
CONCRETE	SLABS ON GROUND	4,000 PSI	0.54
	REMAINING CONCRETE	4,000 PSI	0.52
	TILT-UP WALL PANELS	REFERENCE TILT-UP CONCRETE WAL	L GENERAL NOTES
1.—1	BAR TYPE	YIELD STRENGTH	
REINF.	WELDABLE REBAR	ASTM A-706, GRADE	60
ଅଧିକା	ALL OTHER REBAR	ASTM A-615, GRADE	60
	WELDED WIRE MESH / FABRIC	ASTM A-185, GRADE	65

- ALL CONCRETE SHALL HAVE A MINIMUM SLUMP OF 4" PLUS OR MINUS 1", AND HAVE 2 TO 4% AIR ENTRAINMENT. CONCRETE TO BE USED FOR INTERIOR FLOOR SLABS SHALL CONTAIN ONLY "ENTRAPPED AIR" AND SHALL HAVE NOT MORE THAN 3& MAXIMUM AIR CONTENT. CONCRETE PLACED WITH A PUMP SHALL HAVE A SLUMP OF 5" PLUS OR MINUS 1". BUILDER MAY ELECT TO PROVIDE AN ALTERNATE MIX DESIGN WITH HIGH RANGE WATER REDUCER WITH A HIGHER SLUMP. SUBMIT ALTERNATE MIX DESIGN TO ARCHITECT AND ENGINEER OF RECORD FOR REVIEW.
- 4. CONCRETE SHALL CONTAIN THE MAXIMNUM SIZE AGGREGATE PERMITTED BY ACI UP TO 1-1/2" MAXIMUM. THE GUIDELINES FOR MAXIMUM AGGREGATE SIZE ARE: A. NOT GREATER THAN 1/5TH THE NARROWEST OPENING IN THE FORMS
- C. FLOOR SLABS WHICH ARE 6" AND GREATER SHALL HAVE #467 AND #6 BLENDED COARSE AGGREGATE 5. CONCRETE MIX DESIGN SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 CHAPTER 3., METHOD 1 OR

B. NOT GREATER THAN 1/3 RD THE THE THICKNESS OF THE SLAB

DIPPED GALVANIZED. SEE ELEVATED SLAB PLANS FOR LOCATIONS.

- METHOD 2. SUBMIT BACKUP DATA AS REQUIRED BY CHAPTER 26 OF THE LATEST EDITION OF ACI 318. 6. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL IN ACCORDANCE WITH THE "SCHEDULE OI CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS" BELOW. REINFORCING STEEL FOR ELEVATED EXTERIOR SLABS WITHIN 3'-0" OF THE SLAB EDGE PERIMTER AND WITHIN 12" OF ALL EXTERIOR SLEEVES AND BLOCK-OUTS SHALL BE HOT-
- ALL REINFORCEMENT SHALL BE PLACED WITH THE REQUIRED CONCRETE COVER TO REINFORCEMENT AS NOTED IN THE FOLLOWING "SCHEDULE OF CONCRETE PROTECTION FOR REINFORCEMENT":

<u>PRO</u>	TECTION FOR REINF	ORCEMENT IN CAST-IN-PLACE CON	<u>CRETE</u>
	APPLIC	CATION	CLEAR COVER
(0	CONCRETE CAST AGAINST AND PERMANENTLY	1. ALL APPLICATIONS EXCEPT SLABS ON GROUND	3"
ACE E (SSED)	EXPOSED TO EARTH	2. SLABS ON GROUND - CLEAR TO TOP OF SLAB	1-1/2"
CAST-IN-PLAC CONCRETE (NON-PRESTRESS	CONCRETE EXPOSED TO	1. #6 BARS AND LARGER	2"
ST-IN CONC	EARTH OR WEATHER	2. #5 BARS AND SMALLER	1-1/2"
CAS C	CONCRETE NOT EXPOSED TO EARTH OR WEATHER	1. SLABS, JOISTS, AND WALLS	3/4"
۷)		2. BEAMS, COLUMNS, AND OTHER MEMBERS	1-1/2"
ED C	CONCRETE EXPOSED TO EARTH OR WEATHER	1. SLABS, JOISTS, AND WALLS	1-1/4"
PRESTRESSED OR POST- TENSIONED CONCRETE		2. BEAMS, COLUMNS, AND OTHER MEMBERS	1-1/2"
ESTE DR P ENSI	CONCRETE NOT EXPOSED	1. SLABS, JOISTS, AND WALLS	3/4"
PRI T	TO EARTH OR WEATHER	2. BEAMS, COLUMNS, AND OTHER MEMBERS	1-1/2"
NOTES:			
1. TOLER	ANCE FOR CONCRETE COVER AN	ND REINFORCEMENT LOCATION IS +/- 3/8".	

- 2. TOTAL CLEAR COVER AT EXTERIOR FACE IS THE SCHEDULED CLEAR COVER PLUS THE DEPTH OF ANY REVEAL.
- 8. ALL REINFORCING DETAILS SHALL CONFORM TO "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" ACI 315 LATEST EDITION, UNLESS DETAILED OTHERWISE ON THE STRUCTURAL DRAWINGS.
- 9. CONTRACTOR SHALL REVIEW ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF EMBEDDED ITEMS, SLEEVES, SLAB DEPRESSIONS, SLOPES, ETC. REQUIRED BY OTHER TRADES. THESE ITEMS SHALL BE FURNISHED AND INSTALLED PRIOR TO PLACEMENT OF CONCRETE. COORDINATE BEARING CONDITIONS REQUIRED BY TILT-UP PANEL LIFTING ENGINEER BEFORE PLACING CONCRETE.
- 10. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, ANCHOR BOLTS, INSERTS, ETC. AS REQUIRED BY
- 11. WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS. THE LENGTH OF ANY HOOK, IF REQUIRED, IS NOT INCLUDED. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS AND AT SLAB EDGES.
- 12. CONTRACTOR SHALL PROVIDE SPACERS, CHAIRS, BOLSTERS, ETC. NECESSARY TO SUPPORT REINFORCING STEEL. SUPPORT ITEMS WHICH BEAR ON EXPOSED CONCRETE SURFACES SHALL HAVE ENDS WHICH ARE PLASTIC TIPPED OR
- 13. CONTRACTOR SHALL PROVIDE 3/4" INCH CHAMFER ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS, AND WALLS UNLESS OTHERWISE INDICATED ON THE ARCHITECTURAL DRAWINGS.
- 14. HORIZONTAL KEYWAYS IN CONSTRUCTION JOINTS SHALL BE PROVIDED WITH A DEPTH OF 1 ½ INCHES AND A HEIGHT EQUAL TO ONE-THIRD OF THE MEMBER'S DEPTH. REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS UNLESS OTHERWISE NOTED ON THE DRAWINGS. CONSTRUCTION JOINTS MAY BE USED ONLY AT LOCATIONS SHOWN ON THE DRAWINGS OR AT OTHER LOCATIONS APPROVED BY THE ARCHITECT AND ENGINEER OF RECORD.
- 15. CONTINUOUS RX WATERSTOP SHALL BE PROVIDED ALONG BASE OF WALLS AND ALONG VERTICAL WALL JOINTS AT ALL RETAINING WALLS AND ELEVATOR STEMWALLS. PLACE WATERSTOP IN ACCORDANCE WITH THE MANUFACTURER'S PROCEDURES.
- 16. REINFORCEMENT SPLICES SHALL NOT BE PERMITTED EXCEPT AS DETAILED OR AUTHORIZED BY THE PROJECT STRUCTURAL ENGINEER. WHERE INDICATED, THE MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE CLASS B PER ACI 318, SECTION 25.5 EXCEPT WHERE OTHERWISE NOTED ON THE DRAWINGS. SEE TABLES PROVIDED.
- 17. TESTING LABORATORY SHALL SUBMIT ONE COPY OF ALL CONCRETE TEST REPORTS DIRECTLY TO THE ENGINEER.
- A. THE OWNER SHALL EMPLOY A TESTING LABORATORY TO TAKE AND TEST CONCRETE CYLINDERS, PERFORM SLUMP TESTS, PERFORM TESTS FOR AIR CONTENT, AND TO PERFORM STRENGTH TESTS IN ACCORDANCE WITH ASTM C39.
- B. A MINIMUM OF THREE (3) CYLINDERS SHALL BE TAKEN FOR EACH 50 CU. YD. OF CONCRETE OR FRACTION THEREOF FOR EACH STRENGTH AND TYPE OF CONCRETE BEING CAST ON ANY DAY. C. NO CONCRETE SHALL BE PLACED THAT DOES NOT MEET SLUMP OR AIR CONTENT REQUIREMENTS. ALL TESTS FOR
- AIR CONTENT SHALL BE MADE BY THE PRESSURE METHOD. SLUMP TESTS SHALL BE TAKEN FOR EACH 20 CU. YD. OF CONCRETE BEING PLACED. SLUMP EXCEEDING THE SPECIFIED MAXIMUM, WHEN OCCURRING IN CONSECUTIVE TESTS MADE ON DIFFERENT
- PORTIONS OF THE SAME SAMPLE. WILL BE CAUSE FOR REJECTION OF THAT TRUCKLOAD AND SHALL BE REPORTED TO THE ARCHITECT AND ENGINEER. THE REPLACEMENT OF SUCH CONCRETE WITH THE SPECIFIED CONCRETE SHALL BE COMPLETED AT NO ADDITIONAL EXPENSE TO THE OWNER.
- E. THE CONCRETE TEST REPORTS THALL CONTAIN THE FOLLOWING INFORMATION:
- a. CONCRETE SUPPLIER b. QUANTITY OF CONCRETE REPRESENTED BY SAMPLE
- . LOCATION OF ALL SAMPLES TAKEN d. STRENGTH REQUIREMENT IN PSI AT 28 DAYS
- e. LIST OF ALL MATERIALS USED
- f. ACTUAL SLUMP a. ACTUAL AIR CONTENT PERCENT BY COLUME n. SITE CONDITIONS INCLUDING AIR TEMPTERATURE, WEATHER, ETC.
- i. CONCRETE TEMPERATURE j. CYLINDER WEIGHT AS RECIEVED k. DATE SAMPLE WAS TAKEN I. NUMBER OF DAYS ON PROJECT SITE
- m. DATE TESTED
- n. TEST RESULTS FOR 7 DAUS AND 28 DAYS AGE o. ANY OTHER NECESSARY INFORMATION TO EVALUATE TESTS
- 18. PLACING CONCRETE:
- A. PLACE CONCRETE IN COMPLIANCE WITH ACI 304 AND AS HEREIN SPECIFIED B. BEFORE PLACING AND CONCRETE IN FORMWORK, THOROUGHLY CLEAN AND WASH OUT FORMS WITH WATER.
- C. IF EARTH AT BOTTOM OF FORMS HAS DRIED OUT, RE-WET SO THAT SOIL IS MOIST, BUT FREE OF STANDING WATER
- D. THOROUGHLY WET WOOD FORMS IMMEDIATELY BEFORE PLACING CONCRETE WHERE FORM COATINGS ARE NOT
- E. CONVEY CONCRETE FROM MIXER TO FINAL POSITION BY METHODS WHICH WILL PREVENT SEPARATION OR LOSS OF MATERIALS.
- F. MAXIMUM HEIGHT OF CONCRETE FREE FALL IS 4 FT. (U.N.O.)
- G. REGULATE RATE OF PLACEMENT SO CONCRETE SURFACE IS KEPT LEVEL THROUGHOUT, A MINIMUM BEING PERMITTED TO FLOW FROM ONE AREA TO ANOTHER. USE TREMIE HEADS SPACED AT APPROXIMATELY 10 FT. INTERVALS FOR PLACING CONCRETE IN WALLS. CONTROL RATE OF POUR CONSISTENT WITH FORM DESIGN.
- H. DEPOSIT CONCRETE IN CONTINUOUS OPERATION UNTIL SECTION BEING PLACED HAS BEEN COMPLETED. FORMWORK
- LATERAL, STATIC, AND DYNAMIC LOADS, AND CONSTRUCTION LOADS THAT MIGHT BE APPLIED, UNTIL CONCRETE STRUCTURE CAN SUPPORT SUCH LOADS. B. CONSTRUCT FORMWORK SO CONCRETE MEMBERS AND STRUCTURES ARE OF SIZE, SHAPE, ALIGNMENT, ELEVATION.

E. SUBMIT DIMENSIONED WALL PANEL SHOP DRAWINGS WITH ALL OPENINGS TO ARCHITECT FOR REVIEW.

A. DESIGN, ERECT, SHORE, BRACE, AND MAINTAIN FORMWORK, ACCORDING TO ACI 301, TO SUPPORT VERTICAL,

C. CONSTRUCT FORMS TIGHT ENOUGH TO PREVENT LOSS OF CONCRETE MORTAR.

AND POSITION INDICATED, WITHIN TOLERANCE LIMITS OF ACI 117.

INSTRUCTIONS, BEFORE PLACING REINFORCEMENT.

D. COAT CONTACT SURFACES OF FORMS WITH FORM-RELEASE AGENT, ACCORDING TO MANUFACTURER'S WRITTEN

CONCRETE SLAB ON GROUND NOTES

- THE CONCRETE SLAB ON GROUND FOR THIS PROJECT IS PRESCRIPTIVE. NO STRUCTURAL DESIGN HAS BEEN PROVIDED.
- 2. THE CONCRETE SLAB ON GROUND HAS BEEN SPECIFIED BASED ON THE FOLLOWING ASSUMPTIONS:
- MINIMUM SOIL BEARING PRESSURE OF **2000** PSF B. SOIL CONSTANT "K" VALUE OF <u>150</u> PSI / IN. VALUES MUST BE CONFIRMED BY GEOTECH REPORT. REFERENCE "FOUNDATION AND GEOTECHNICAL NOTE" #2
- THE SLAB ON GRADE CAN ACCOMMODATE A WORKING UNIFORM LOAD OF 500 PSF BASED ON SLAB THICKNESS AND
- 4. THE FOLLOWING GUIDELINES SHALL APPLY TO THE LAYOUT OF CONTRACTION / CONTROL JOINTS IN THE SLAB. THESE JOINTS SHALL BE PROVIDED AT THE FOLLOWING SPACINGS (MAXIMUM):
- A. 14'-0" ON CENTER FOR 6" SLABS AND THINNER B. 15'-0" ON CENTER FOR 7" SLABS AND THICKER
- 5. THE SLAB ON GRADE SHALL MEET THE FOLLOWING FLATNESS / LEVELNESS (FF / FL) REQUIREMENTS
- A. TYPICAL FLOOR SLAB: FF 45 / FL 35
- PROVIDE ALL LABOR, PRUDUCTS, AND EQUIPMENT REQUIRED TO PROPERLY INSTALL AN UNDERSLAB VAPOR RETARDER UNDER ALL INTERIOR CONCRETE FLOOR SLABS ON GROUND. REFER TO DRAWINGS FOR REQUIRED LOCATIONS.
- VAPOR RETARDER MATERIAL SHALL BE A MULTILAYER POLYETHYLENE SHEET MATERIAL CONFORMING TO ASTM E 1745. CLASS "A", FOR A 15 MIL THICKNESS. VAPORT RETARDER SHALL HAVE A WATER VAPOR PERMEANCE OF LESS THAN 0.0254 PERMS ACCORDING TO ASTM F 1249. STEGO WRAP CLASS A VAPOR RETARDER OR APPROVED EQUAL
- PROVIDE ALL REQUIRED ACCESSORY MATERIALS BY THE VAPOR RETARDER, INCLUDING SEAM TAPE AND MASTIC ACCESSORY MATERIALS SHALL HAVE A WATER VAPOR PERMEANCE OF 0.3 PERMS OR LOWER ACCORDING TO ASTM E 96.
- INSTALLER SHALL PROCEED WITH APPLICATION OF THE VAPOR RETARDER ONLY AFTER SUBSTRATE CONSTRUCTION AND PENETRATING WORK HAVE BEEN COMPLETED AND ANY UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED. 10. INSTALLER SHALL COMPLY WITH ALL VAPORT RETARDER MANUFACTURER'S WRITTEN INSTALLATION INSTRUCTIONS AS
- WELL AS THE REQUIREDMENTS OF ASTM E 1643. 11. UNROLL THE VAPOR RETARDER MATERIAL WITH THE LONGEST DIMENSION PARALLEL WITH THE DIRECTION OF THE CONCRETE POUR. LAP OVER FOOTINGS OR SEAL TO FOUNDATION WALLS.
- 12. OVERLAP ALL JOINTS 6 INCHES AND SEAL WITH MANUFACTURER PROVIDED SEAM TAPE
- SEAL ALL PENETRATIONS ACCORDING TO THE MANUFACTURER'S WRITTEN INSTRUCTIONS
- 14. NO PENETRATION OF VAPOR RETARDER MATERIAL IS ALLOWED EXCEPT FOR REINFORCING STEEL AND PERMANENT
- 15. $\,$ INSTALLER SHALL REPAIR DAMAGED AREAS BY CUTTING RECTANGULAR PATCHES OF THE VAPOR RETARDER MATERIAI OVERLAPPING THE DAMAGED AREA 6 INCHES AND TAPING ALL FOUR SIDES WITH MANUFACTURER PROVIDED SEAM TAPE

TILT-UP CONCRETE WALL SYSTEM NOTES

STRUCTURAL CONCRETE," - ACI 318 LATEST EDITION, AND "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS," - ACI 301 LATEST EDITION.

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE PUBLICATIONS "BUILDING CODE REQUIREMENTS FOR

ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS IN ACCORDANCE WITH THE FOLLOWING "SCHEDULE OF CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS":

SCHE	DULE OF CAST-IN-PLA	ACE CONCRETE CONSTRUCTION	ON MATERIALS
	PANEL TYPE	28-DAY COMPRESSIVE STRENGTH, fc	MAX w/c RATIO
	7-1/4" PANELS	4,000 PSI	0.50
CONCRETE			
⊢ய்ய	BAR TYPE	YIELD STRENGTH	
REINF. STEEL	WELDABLE REBAR	ASTM A-706, GRADE 6	60
ଅଧାରୀ	ALL OTHER REBAR	ASTM A-615, GRADE 6	60
	WELDED WIRE MESH / FABRIC	ASTM A-185, GRADE 6	65

- 3. ALL CONCRETE SHALL BE PLACED IN A CONTINUOUS OPERATION AND CONSOLIDATED USING MECHANICAL VIBRATION. POUR STOPS AND CONSTRUCTION JOINTS WILL NOT BE APPROVED WITHIN A SINGLE PANEL. CONCRETE STRENGTH AT PANEL LIFTING SHALL BE EQUAL TO 75% OF SPECIFIED 28 DAY CONCRETE STRENGTH, 3,000 PSI MINIMUM.
- 4. ALL REINFORCEMENT SHALL BE NEW HIGH-STRENGTH BILLET STEEL, DEFORMED AS PER ASTM A-305 AND CONFORMING TO MATERIAL PROPERTIES AND STRENGTHS AS NOTED IN THE "SCHEDULE OF CAST-IN-PLACE CONCRETE CONSTRUCTION MATERIALS." ALL REINFORCEMENT TO BE DETAILED & FABRICATED IN ACCORDANCE WITH "MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCED CONCRETE STRUCTURES" AND ACI 318, LATEST EDITION.
- REINFORCEMENT SHALL BE POSITIONED, AND SUPPORTED IN ACCORDANCE WITH CRSI "MANUAL FOR PLACING" REINFORCING BARS."
- 6. ALL REINFORCEMENT SHALL BE PLACED WITH THE REQUIRED CONCRETE COVER TO REINFORCEMENT AS NOTED IN THE FOLLOWING "SCHEDULE OF CONCRETE PROTECTION FOR REINFORCEMENT": DDOTECTION FOR REINFORCEMENT IN THE LID WALL CONCRETE

PRO	<u>JIECTION FOR REIN</u>	FORCEMENT IN TILT-UP WALL CONC	REIE
	APPLI	CATION	CLEAR COVER
1 4	CONCRETE EXPOSED TO EARTH OR WEATHER	1. #6 BARS AND LARGER	1-1/4"
o WA RETI	(EXTERIOR FACE) CONCRETE NOT EXPOSED TO EARTH OR WEATHER	2. #5 BARS AND SMALLER	1"
TILT-UP WALL CONCRETE		1. #6 BARS AND LARGER	3/4"
₽°	(INTERIOR FACE)	2. #5 BARS AND SMALLER	3/4"
NOTES:			

- TOLERANCE FOR CONCRETE COVER AND REINFORCEMENT LOCATION IS +/- 3/8" TOTAL CLEAR COVER AT EXTERIOR FACE IS THE SCHEDULED CLEAR COVER PLUS THE DEPTH OF ANY REVEAL.
- 7. STRUCTURAL STEEL PLATES AND SHAPES TO BE ASTM A-36 MATERIAL, AND SHALL BE DETAILED AND FABRICATED IN ACCORDANCE WITH AISC SPECIFICATIONS.
- 8. ALL CONCRETE ANCHORS SHALL BE EMBEDDED A MINIMUM OF 7" INTO CONCRETE FOUNDATIONS UNLESS OTHERWISE NOTED. ALL ANCHORS MUST BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS.
- 9. LOCATION OF JOISTS, BEAMS AND OTHER EMBED REQUIREMENTS MUST BE VERIFIED BY THE GENERAL CONTRACTOR PRIOR TO FABRICATION AND PLACEMENT OF CONCRETE. 10. RUSTICATION FORMING MATERIAL (FORM LINER) MATERIAL SHALL BE NEW AND IN UNUSED CONDITION UNLESS FORM
- WORK IS DESIGNED FOR REUSE. 11. WHERE "RUBBER BEARING PAD" IS SPECIFIED AT TILT-UP WALL SPANDREL BEARING LOCATIONS, THESE BEARING PADS
- SHALL MEET OR EXCEED THE FOLLOWING CRITERIA: A. BEARING PADS SHALL BE AASHTO M 251 PLAIN, VULCANIZED, 100 PERCENT NEOPRENE ELASTOMER, MOLDED TO SIZE

ALL BURRS, HONEYCOMBING, AND POCKETS SHALL BE REMOVED FROM PANELS IMMEDIATELY AFTER PANEL ERECTION

OR CUT FROM A MOLDED SHEET B. BEARING PADS SHALL BE TYPE A SHORE DUROMETER HARDNESS OF 60 TO 70, ASTM D 2240; MINIMUM TENSILE STRENGTH 2250 PSI, ASTM D 412

BEARING PADS SHALL BE 3/4" THICK (UNLESS NOTED OTHERWISE)

- BEARING PADS SHALL BE BLACK STACK CASTING OF TILT-UP WALL PANELS SHALL NOT BE PERMITTED WITHOUT PRIOR WRITTEN APPROVAL OF THE INEER. THIS APPROVAL SHALL BE LIMITED TO SPECIFIC PANELS.
- UNLESS OTHERWISE INDICATED ON PLANS, ALL PANELS SHALL RECEIVE A STEEL TROWELED FINISH ON THE INTERIOR SURFACE. THE EXTERIOR SURFACE OF THE PANELS SHALL BE CAST AGAINST THE FUTURE FLOOR SLAB, OR TEMPORARY CASTING BED, WHICH SHALL HAVE A STEEL TROWELED FINISH. HOLES CREATED BY ERECTION HARDWARE AND LIFTING DEVICES SHALL BE PATCHED SMOOTH. PREFABRICATED INSERT
- PLUGS ARE NOT ACCEPTABLE. 16. TYPICAL PANEL REINFORCING SHALL CONTINUE UNINTERRUPTED THROUGH "KNOCK OUT AREAS" AS NOTED ON PANEL ELEVATIONS. ADDITIONAL REINFORCING AROUND THE "KNOCK OUT AREA" FOR FUTURE PANEL OPENING SHALL ALSO BE
- 17. HOLES LEFT IN THE PERMANENT SLAB ON GROUND DUE TO PANEL BRACES SHALL BE FILLED PARTIALLY WITH A NON-SHRINK GROUT AND TOPPED WITH A MINIMUM 2" LAYER OF DAYTON SUPERIOR (J-50) TWO-PART EPOXY FILL OR APPORVED EQUIVALENT. THE EPOXY TOP LAYER SHALL BE GROUND SMOOTH.
- 18. SUBMIT SIGNED AND SEALED SHOP DRAWINGS AND CALCULATIONS FOR LIFTING INSERTS. PROVIDE ADDITIONAL WALL REINFORCEMENT, STRONG BACKS, ETC. AS REQUIRED FOR LIFTING, ERECTING, AND BRACING CONCRETE PANELS.
- 19. SUBMIT DETAILED SHOP DRAWINGS INCLUDING THE FOLLOWING:
- A. PANEL INSERTS B. EMBED PLATES
- . SCHEDULED REINFORCEMENT). ADDITIONAL REINFOREMENT FOR PANEL ERECTION
- STRONG BACKS AND OTHER REQUIRED ERECTION HARDWARE PANEL DIMENSIONS INCLUDING ALL OPENINGS AND PENETRATIONS
- G. SLAB LAYOUT SHOWING LOCATION OF PANEL CASTING H. SHOP DRAWINGS AND RELATED SUBMITTALS SHALL MATCH PANEL IDENTIFICATION NUMBERS SHOWN ON STRUCTURAL DRAWINGS, UNLESS APPROVED OTHERWISE
- 20. PROVIDE SIGNED AND SEALED SHOP DRAWINGS FOR TEMPORARY TILT-UP WALL PANEL BRACING. 21. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR PANEL DIMENSIONS, SIZE AND LOCATION OF OPENINGS, REVEALS

STRUCTURAL STEEL NOTES

STRUCTURAL SHAPE

ANGLES, PLATÈS, CHANNEL SHAPES

ANCHOR RODS (ANCHOR BOLTS)

STEEL TUBE - SQUARE / RECT

HEADED SHEAR CONNECTORS

WIDE FLANGE (W-)

PIPE (STD, XS, XXS)

STEEL TUBE - ROUND

HIGH STRENGTH BOLTS

HARDENED WASHERS

- 1. ALL STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," - AISC 360, LATEST EDITION.
- WELDED CONNECTIONS SHALL CONFORM TO THE LATEST EDITION CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1

ASTM A992

ASTM A36

ASTM A325

ASTM F436

- 3. ALL FABRICATION AND ERECTION WORK SHALL BE PERFORMED BY AISC CERTIFIED FABRICATORS AND ERECTORS
- 4. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING MATERIAL REQUIREMENTS, UNLESS NOTED OTHERWISE:
 - MATERIAL SPEC YIELD STRENGTH Fy = 50 KSI $F_y = 36 \text{ KSI}$ ASTM F1554, GRADE 36 Fy = 36 KSIFy = 55 KSI - S1 SUPPLEMENT ASTM F1554. GRADE 55 ASTM A53, GRADE B $F_V = 36 \text{ KSI}$ ASTM A500, GRADE C $F_y = 50 \text{ KSI}$ ASTM A500, GRADE C $F_V = 46 \text{ KSI}$
- ASTM A108, TYPE B, GRADE 1010 1020 5. ALL STEEL SHAPES, PLATES, FASTENERS, ETC. WHICH ARE EXPOSED TO WEATHER SHALL BE HOT-DIPPED GALVANIZED.
- ALL STEEL SHAPES, PLATES, FASTENERS, ETC. WHICH ARE EXPOSED TO SOIL SHALL BE ENCASED IN CONCRETE 7. ALL INTERIOR STRUCTURAL STEEL SHALL BE PAINTED WITH A RUST INHIBITIVE PRIMER. DO NOT USE PRIMER AT MEMBERS THAT ARE TO RECEIVE SPRAY-ON FIRE PROOFING. COORDINATE PRIMER WITH ARCHITECT AND OWNER'S
- PAINTING REQUIREMENTS. 8. AND APPORVED LICENSED TESTING AGENCY SHALL VISUALLY INSPECT ALL WELDS, BOLTED CONNECTIONS, METAL DECK ATTACHMENT, AND OTHER STRUCTURAL STEEL CONNECTIONS.
- 9. FIELD STRUCTURAL STEEL TO BE INSPECTED BY QUALIFIED INSPECTORS APPROVED BY THE STRUCTURAL ENGINEER FIELD INSPECTION REPORT SHALL BE FILED WITH THE STRUCTURAL ENGINEER WITHIN 5 DAYS OF THE DATE OF THE INSPECTION. INSPECTORS MUST BE NOTIFIED OF ALL PHASES OF CONSTRUCTION BY THE GENERAL CONTRACTOR
- 10. ALL WELDED CONNECTIONS SHALL BE COMPLETED WITH E70XX ELECTRODES. SHOP AND FIELD WELDS SHALL BE MADE BY APPROVED CERTIFIED WELDERS AND SHALL CONFORM TO THE AMERICAN WELDING SOCIETY CODE OF BUILDINGS AWS D1.1. WELDS SHALL DEVELOP THE FULL STRENGTH OF THE MATERIALS BEING WELDED UNLESS NOTED OTHERWISE. ALL WELDS SHALL BE IN ACCORDANCE WITH THE STRUCTURAL WELDING CODE (ANSI/AWS D1.1).
- 11. TOUCH UP FIELD WELDS AND ANY DAMAGED AREAS OF PAINT IN FIELD AFTER WELDED. USE TWO COATS OF COLD

GALVANIZING COMPOUND PAINT FOR TOUCH UP OF GALVANIZED STEEL

- 12. ALL HSS AND PIPE SHAPES SHALL HAVE CLOSURE PLATES AND CONTINUOUS WELDS TO SEAL THE SECTIONS. 13. BEFORE PROCEEDING WITH ERECTION, AND WITH THE STEEL ERECTOR PRESENT, VERIFY ELEVATIONS OF CONCRETE
- PROCEED WITH ERECTION UNTIL UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED. 14. NON-METALLIC, NON-SHRINK GROUT UNDER ALL COLUMN BASE PLATES AND BEAM BEARING PLATES SHALL CONSIST OF A PREMIXED PRODUCT COMPLYING WITH ALL REQUIREMENTS OF CRD-C621, ASTM C827, AND C109. GROUT STRENGTH TO

AND MASONRY BEARING SURFACES AND LOCATIONS OF ANCHORAGES FOR COMPLIANCE WITH REQUIREMENTS. DO NOT

BE 6000 PSI (MIN) WHEN BEARING ON 3000 PSI CONCRETE, AND 8000 PSI (MIN) WHEN BEARING ON 4000 PSI CONCRETE.

15. SPLICING OF STRUCTURAL STEEL WHERE NOT DETAILED IS NOT PERMITTED WITHOUT WRITTEN APPROVAL OF ENGINEER.

OPEN WEB STEEL JOIST NOTES

JOIST LOADING TABLES PER THE STEEL JOIST INSTITUTE.

DISCONTINUOUS FOR ANY REASON, AND AS FOLLOWS:

- DESIGN. DETAILING. FABRICATION. AND ERECTION OF STEEL JOISTS AND JOIST GIRDERS SHALL BE IN ACCORDANCE WITH "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS," - AISC 360, LATEST EDITION AND WITH THE LATEST CODES AND STANDARDS OF THE STEEL JOIST INSTITUTE, SJI.
- 2. STEEL JOIST AND JOIST GIRDER MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE. 3. JOISTS SHALL BE DESIGNED FOR THE COMBINED DEAD, LIVE, AND WIND LOADS AS NOTED IN THE LOAD TABLES AND AS NOTED ON PLAN. IN ALL CASES, JOISTS SHALL NOT BE DESIGNED FOR LESS LOAD THAN PRESCRIBED IN THE STANDARD
- PROVIDE UPLIFT BRIDGING AND STANDARD JOIST BRIDGING IN ACCORDANCE WITH THE LATEST SJI SPECIFICATIONS

5. PROVIDE X-BRACING IN ACCORDANCE WITH THE LATEST SJI SPECIFICATIONS, WHERE ANY BRIDGING LINE IS

- A. AT ALL HORIZONTAL BRIDGING LINES INCLUDING UPLIFT BRIDGING AT INTERVALS NOT TO EXCEED 200 FT.
- (EITHER DIRECTLY ADJACENT OR NEXT BAY). C. BOTH SIDES OF INTERIOR W BEAMS WHERE JOISTS ARE LOCATED (EITHER DIRECTLY ADJACENT OR NEXT BAY).

6. WELD ALL STEEL JOISTS TO SUPPORTING STRUCTURAL STEEL MEMBERS AS SHOWN ON THE DRAWINGS, ACCORDING TO

B. AT ALL HORIZONTAL BRIDGING LINES INCLUDING UPLIFT BRIDGING WHERE TERMINATING AT OUTSIDE (EDGE) BEAMS

A. MINIMUM JOIST WELD = 3/16" FILLET, 2" LONG EACH SIDE OR 1/8" FILLET, 3" LONG EACH SIDE

SJI AS MINIMUM, BUT NOT LESS THAN THE FOLLOWING UNLESS APPROVED BY EOR:

B. MINIMUM JOIST GIRDER WELD = 1/4" FILLET, 4" LONG EACH SIDE

SUSPENDED MEDICAL EQUIPMENT.

SLOPE EXCEEDS 1/4" PER 12 INCHES.

WITH THE AWS-D.1 UNLESS NOTED OTHERWISE

- JOIST CAMBER MAY BE USED IN COMPUTING THE JOIST DEFLECTION. LIVE LOAD DEFLECTION MUST NOT EXCEED SPAN/360 UNDER UNIFORM LOADING. CAMBER MUST NOT BE CONSIDERED FOR CONCENTRATED LOADS FROM
- B. BOTTOM CHORD OF THE K AND KCS SERIES JOIST SHALL BE FABRICATED OF TEES OR ANGLES, IN LIEU OF RODS. 9. COORDINATE THE EXACT LOCATION OF ALL MECHANICAL AND ARCHITECTURAL OPENINGS WITH THE MECHANICAL AND ARCHITECTURAL DRAWINGS AS WELL AS WITH OTHER SUBCONTRACTORS PRIOR TO FABRICATION OF JOISTS.
- THE JOISTS SHALL BE LOCATED AT THE JOIST PANEL POINTS, OR PROVIDE JOIST STIFFENERS. JOIST ENGINEER SHALL DESIGN ALL JOISTS FOR A MINIMUM 150 LB BEND-CHECK FOR BOTH TOP AND BOTTOM CHORDS UNLESS A GREATER LOAD

10. ALL HANGERS SUPPORTING MECHANICAL EQUIPMENT, PIPES, AND OTHER CONCENTRATED LOADS TO BE SUPPORTED BY

THE BOTTOM CHORD SHALL BE EXTENDED TO THE COLUMN. IF NO JOIST IS PRESENT AT COLUMN CENTERLINE, THE CLOSEST ADJACENT JOISTS ON EACH SIDE OF THE COLUMN SHALL FOLLOW THIS REQUIREMENT.

12. STEEL JOISTS AND JOIST GIRDERS SHALL HAVE MANUFACTURER'S STANDARD BEVELED ENDS OR SLOPED SEATS IF ROOF

11. ALL JOISTS ON COLUMN CENTERLINES SHALL BE SECURED BY 1/2" DIAMETER A325 BOLTS AT THE TOP CHORD BEARING.

13. STEEL JOISTS SHALL BEAR 4" MINIMUM ON MASONRY / CONCRETE, AND 2-1/2" MINIMUM ON STEEL U.N.O. JOISTS BEARING ON MASONRY / CONCRETE SHAL BEAR ON AN EMBEDDED STEEL PLATE.

14. WELDING OF JOISTS BEARING ON STEEL SHALL BE AS SPECIFIED BY THE SJI AND WELDING SHALL BE IN ACCORDANCE

LOADS AND THE WIND PRESSURE ZONE PLAN. PROVIDE ADDITIONAL BRIDING AS REQUIRED. SHOW ALL BRIDGING AND

- 15. STEEL JOISTS AND JOIST GIRDERS SHALL BE PRIMED PAINTED WITH ONE COAT OF GRAY PRIMER MEETING THE MINIMUM REQUIREMENTS OF SSPC-PAINT 25 OR STEEL STRUCTURES PAINTING COUNCIL SPECIFICATION 15-68T, TYPE I. 16. JOIST MANUFACTURER SHALL DESIGN ROOF JOISTS FOR THE UPLIFT WIND PRESSURE INDICATED IN TABLE 1: DESIGN
- DETAILS ON THE JOIST SHOP DRAWINGS. 17. JOIST MANUFACTURER SHALL PROVIDE A WRITTEN STATEMENT VERIFYING THAT ALL STEEL JOISTS AND JOIST GIRDERS ARE DESIGNED IN ACCORDANCE WITH ALL THE DESIGN REQUIREMENTS OF THE PROJECT. THIS STATEMENT SHALL BE

SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED.

METAL ROOF DECK NOTES

2. STEEL DECK PROFILE SHALL CONFORM TO FACTORY MUTUAL REQUIREMENTS

PATTERN SHOWN ON THE CONTRACT DRAWINGS

DETERMINED BY THE ARCHITECT OR ENGINEER.

- DETAILING, FABRICATION, AND ERECTION OF STEEL DECK SHALL BE IN ACCORDANCE WITH THE LATEST STEEL DECK ISTITUTE SPECIFICATIONS, AWS, AND CONTRACT DOCUMENTS. DECK SHALL CONFORM TO "BASIC DESIGN SPECIFICATIONS" AS ADOPTED BY THE STEEL DECK INSTITUTE, SDI.
- AND GRAY TOP SIDE STEEK DECK CONFORMING TO ASTM A1008 OR ASTM A1039 WITH MINIMUM YIELD STRESS OF 50 KSI. REFERENCE DRAWINGS FOR REQUIRED DECK STRENGTH. DECK FINISH SHALL BE SHOP PRIMED WITH BAKED-ON, LEAD-AND CHROMATE-FREE RUST-INHIBITIVE PRIMER COMPLYING WITH PERFORMANCE REQUIREMENTS OF SSPC-PAINT 25.

3. METAL ROOF DECK SHALL BE MINIMUM 1-1/2" DEEP 22 GA., TYPE B (AS IDENTIFIED BY SDI) PAINTED WHITE UNDERSIDE

OVER THE SUPPORTS. THE DECK SHALL BE ATTACHED TO THE SUPPORTS, AND THE SIDE LAP OF ADJACENT UNITS IN THE

- DECK SUPPLIER SHALL PROVIDE ANY MISCELLANEOUS CLOSURE PIECES, POUR STOPS, DRAIN SUMP PANS, ETC. TO COMPLETE PROJECT. MISCELLANEOUS ITEMS SHALL MATCH (AT A MINIMUM) THE STEEL DECK FINISH AND THICKNESS THE DECK SHALL BE PLACED ON THE SUPPORTING FRAMEWORK WITH A MINIMUM END LAP OF TWO INCHES CENTERED
- 6. ALL ROOF DECK OPENINGS 12" DIAMETER OR LARGER ARE TO HAVE SUPPORT ANGLES PER TYPICAL DECK OPENING DETAIL, INCLUDING OPENINGS FOR ROOF SUMP PANS.
- REINFORCEMENT PER TYPICAL DECK REINFORCEMENT DETAIL. 8. NO LOADS SHALL BE HUNG FROM THE ROOF DECK.

7. ALL ROOF DECK OPENINGS 6" DIAMETER OR LARGER (UP TO 12" DIAMETER) SHALL HAVE LIGHT GAUGE DECK

- 9. ROOF DECK SHALL BE LAID OUT SUCH THAT DECKING SHALL SPAN THREE SPANS WITHOUT INTERRUPTION.
- 11. PUDDLE WELDS SHALL BE AT LEAST 5/8" EFFECTIVE DIAMETER OR AN ELONGATED WELD HAVING AN EQUAL PERIMETER. FILLET AND SEAM WELDS, WHEN USED, SHALL BE A MINIMUM OF 1-1/2" LONG. WELD METAL SHALL PENETRATE ALL LAYERS OF DECK MATERIAL AT END LAPS AND SIDE JOINTS AND HAVE SOLID FUSION TO THE SUPPORTING MEMBERS.

10. DECK AND SUPPORTING MEMBERS DAMAGED BY EXCESS WELDING HEAT SHALL BE REPAIRED OR REPLACED AS

12. STRUCTURAL DRAWINGS HAVE AN ALTERNATE ATTACHMENT METHOD USING POWER ACTUATED FASTENERS AND SCREWS. THIS ATTACHMENT METHOD MAY BE USED AS AN ALTERNATE TO WELDING.



ARCHITECTUR

135 W Central Blvd., Suite 400

TEL: 407.363.6136

AA26001097

Orlando, Florida 32801

LICENSE No 74693 STATE OF CORIDA

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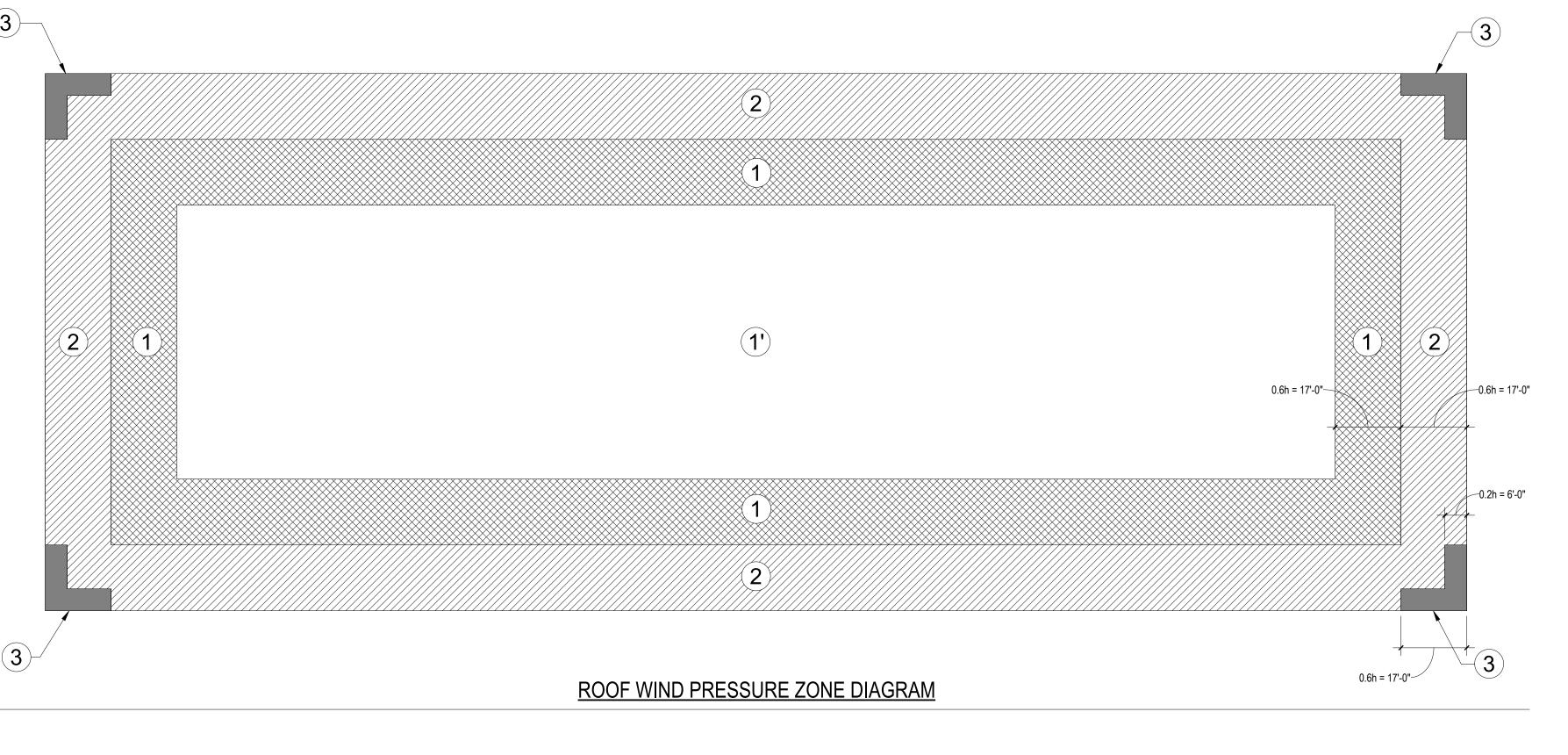
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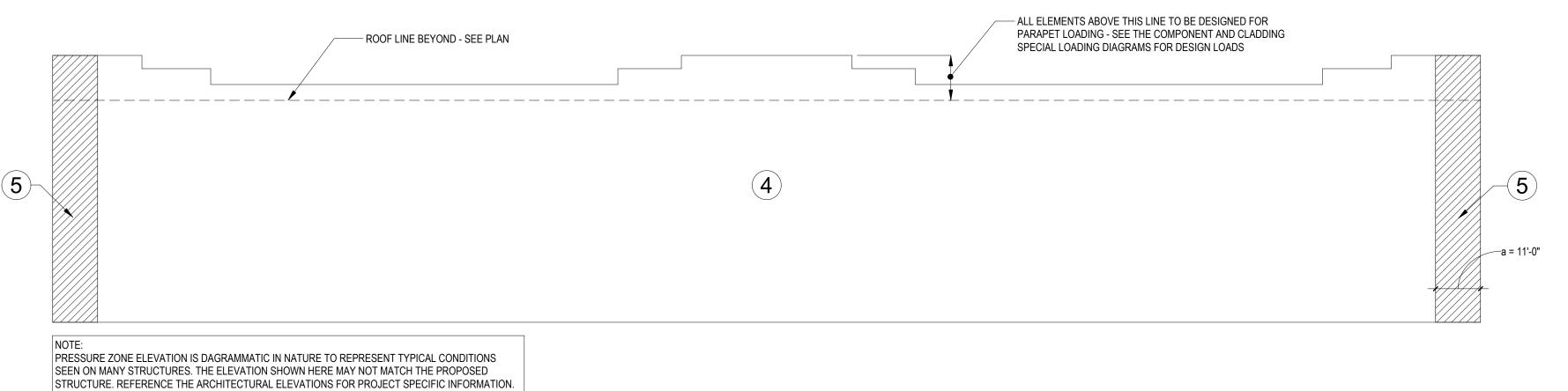
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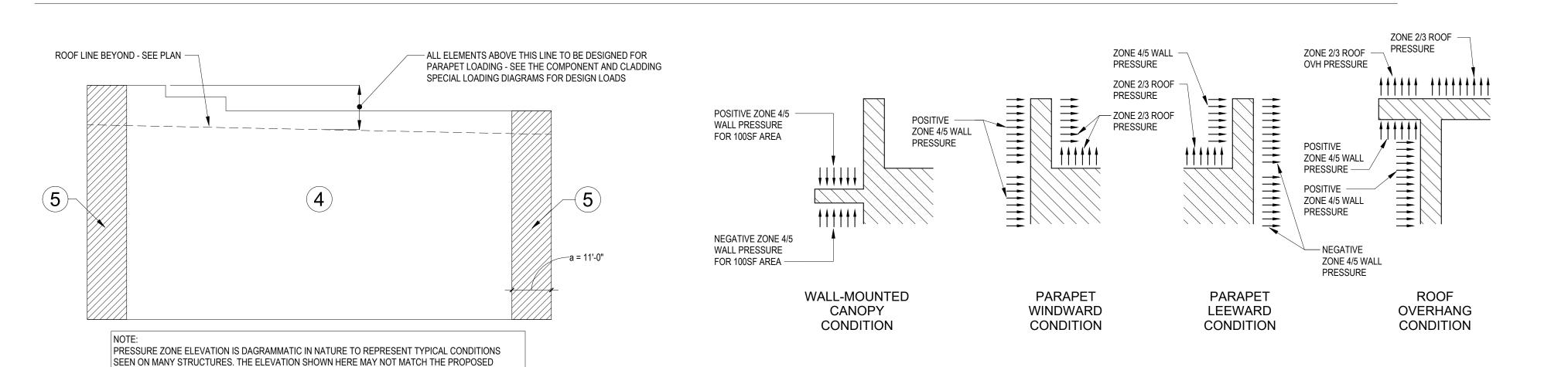
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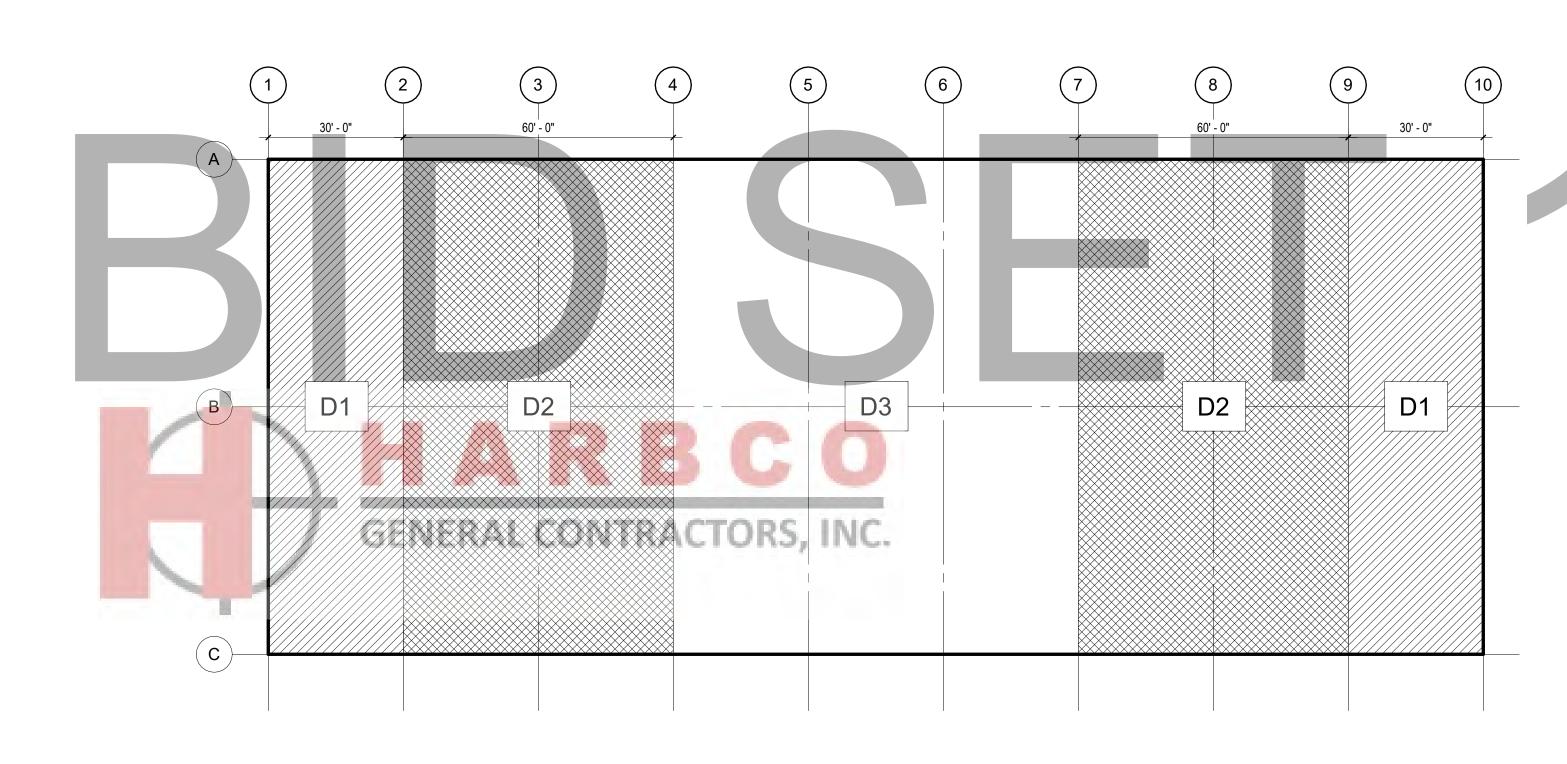




SIDE WALL WIND PRESSURE ZONE DIAGRAM



END WALL WIND PRESSURE ZONE DIAGRAM COMPONENTS AND CLADDING SPECIAL LOADING DIAGRAMS



ROOF DECKING LAYOUT PLAN

STRUCTURE. REFERENCE THE ARCHITECTURAL ELEVATIONS FOR PROJECT SPECIFIC INFORMATION.

TABLE 1: D	ESIGN LC	ADS & DE	SIGN CRI	<u>TERIA</u>			
DEAD LOADS							
BUILDING COMPONENT	TYPICAL ROOF OVER WAREHOUSE	TYPICAL ROOF OVER OFFICE SPACE	TYPICAL MEZZ FLOOR OVER EQUIP ROOMS	NOT USED			
TPO ROOFING	2.0 PSF	2.0 PSF	PSF	PSF			
INSULATION	2.0 PSF	2.0 PSF	PSF	PSF			
METAL DECK	2.0 PSF	2.0 PSF	PSF	PSF			
STEEL JOISTS	3.0 PSF	3.0 PSF	PSF	PSF			
FIRE SPRINKLERS	1.5 PSF	2.0 PSF	PSF	PSF			
MECHANICAL	1.5 PSF	2.0 PSF	PSF	PSF			
CEILING	PSF	2.0 PSF	PSF	PSF			
SOLAR PANELS	PSF	PSF	PSF	PSF			
MISCELLANEOUS	3.0 PSF	3.0 PSF	PSF	PSF			
STRUCT FLOOR SLAB	PSF	PSF	PSF	PSF			
FLOORING	PSF	PSF	PSF	PSF			
TOTAL DEAD LOAD	15.0 PSF	18.0 PSF	PSF	PSF			
	LIV	E LOADS					
ROOF LIVE LOAD	20.0 PSF	20.0 PSF	PSF	PSF			
LIVE LOAD	PSF	PSF	PSF	PSF			
ROOF CONCENTRATED	150 LBS	150 LBS	LBS	LBS			
FLOOR CONCENTRATED	LBS	LBS	LBS	LBS			

WIND LOAD CRITERIA					
BASIC WIND SPEED (ULT)	139 MPH	BUILDING RISK CATEGORY	II		
BASIC WIND SPEED (ASD)	108 MPH	EXPOSURE CATEGORY	С		
VELOCITY PRESSURE, qh (ULT)	40.7	ENCLOSURE CLASSIFICATION	ENCLOSED		
VELOCITY PRESSURE, qh (ASD)	31.5	INTERNAL PRESSURE COEFFICIENT	+/- 0.18		

ROOF LIVE LOADS MAY BE REDUCED, WHERE APPLICABLE, PER FLORIDA BUILDING CODE SEC 1607.12.2.1. REDUCED UNIFORM LIVE LOAD SHALL NOT BE LESS THAN 12.0 PSF.
 CONCENTRATED ROOF LOADS OVER STEEL JOIST ROOFS SHALL BE APPLIED AS BEND CHECK LOADS TO THE ROOF JOISTS AT THE TOP AND BOTTOM CHORDS (NOT SIMULTANEOUSLY).

TABLE 2: STRENGTH DESIGN (ULTIMATE) WIND PRESSURES

POSITIVE WIND PRESSURES ON WALLS & WALL OPENINGS

WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND PRESSURE ZONE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE 4	44.0 PSF	42.0 PSF	39.4 PSF	37.5 PSF	35.5 PSF
WALL ZONE (5)	44.0 PSF	42.0 PSF	39.4 PSF	37.5 PSF	35.5 PSF
NEGATIVE	WIND PRE	SSURES ON	WALLS & W	ALL OPENIN	IGS
WIND DDECCUDE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND PRESSURE ZONE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE 4	-47.6 PSF	-45.7 PSF	-43.1 PSF	-41.2 PSF	-39.2 PSF
WALL ZONE (5)	-58.6 PSF	-54.7 PSF	-49.6 PSF	-45.7 PSF	-41.8 PSF
POSITIVE	WIND PRES	SURES ON F	ROOFING & I	ROOF FRAM	ING
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE (1')	19.5 PSF	18.3 PSF	16.7 PSF	16.0 PSF	16.0 PSF
ROOF ZONE ①	19.5 PSF	18.3 PSF	16.7 PSF	16.0 PSF	16.0 PSF
ROOF ZONE ②	19.5 PSF	18.3 PSF	16.7 PSF	16.0 PSF	16.0 PSF
ROOF ZONE ③	19.5 PSF	18.3 PSF	16.7 PSF	16.0 PSF	16.0 PSF
NEGATIVE	WIND PRES	SURES ON	ROOFING &	ROOF FRAM	IING
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND FRESSURE ZUNE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE (1')	-44.0 PSF	-44.0 PSF	-44.0 PSF	-44.0 PSF	-37.8 PSF
ROOF ZONE ①	-76.5 PSF	-71.5 PSF	-64.8 PSF	-59.8 PSF	-54.7 PSF
ROOF ZONE ②	-100.9 PSF	-94.5 PSF	-85.9 PSF	-79.4 PSF	-72.9 PSF
ROOF ZONE ③	-137.6 PSF	-124.6 PSF	-107.4 PSF	-94.5 PSF	-81.5 PSF

NOTES

WIND PRESSURES IN THE TABLES ABOVE ARE BASED ON CALCULATIONS FROM ASCE 7-16.
 OVERHANG PRESSURES IN THE TABLES ABOVE SHALL APPLY TO ALL ROOFS OVER BALCONIES, BREEZEWAYS, AND COVERED ENTRIES. COORDINATE WITH ARCHITECTURAL DRAWINGS.
 PROVIDE IMPACT RESISTANT GLAZING AS REQUIRED FOR WIND BORNE DEBRIS PER FLORIDA BUILDING CODE.
 MAXIMUM ALLOWABLE DEAD LOADS TO BE USED TO RESIST UPLIFT SHALL BE AS FOLLOWS:

 NET UPLIFT = ULTIMATE UPLIFT - 10 PSF DEAD LOAD

B. NET UPLIFT = ALLOWABLE UPLIFT - 6 PSF DEAD LOAD

TABLE 3: ALLOWABLE STRESS (ASD) WIND PRESSURES

POSITIVE	WIND PRES	SSURES ON	WALLS & WA	ALL OPENIN	GS
WIND DDECCUDE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND PRESSURE ZONE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE 4	26.4 PSF	25.2 PSF	23.7 PSF	22.5 PSF	21.3 PSF
WALL ZONE (5)	26.4 PSF	25.2 PSF	23.7 PSF	22.5 PSF	21.3 PSF
NEGATIVE	E WIND PRE	SSURES ON	WALLS & W	ALL OPENIN	IGS
WIND DDFOOLDF ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND PRESSURE ZONE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
WALL ZONE 4	-28.6 PSF	-27.4 PSF	-25.9 PSF	-24.7 PSF	-23.5 PSF
WALL ZONE (5)	-35.2 PSF	-32.8 PSF	-29.7 PSF	-27.4 PSF	-25.1 PSF
POSITIVE	WIND PRES	SURES ON F	ROOFING & F	ROOF FRAM	ING
WIND PRESSURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	11.7 PSF	11.0 PSF	10.0 PSF	9.6 PSF	9.6 PSF
ROOF ZONE ①	11.7 PSF	11.0 PSF	10.0 PSF	9.6 PSF	9.6 PSF
ROOF ZONE ②	11.7 PSF	11.0 PSF	10.0 PSF	9.6 PSF	9.6 PSF
ROOF ZONE ③	11.7 PSF	11.0 PSF	10.0 PSF	9.6 PSF	9.6 PSF
NEGATIVE	WIND PRES	SURES ON	ROOFING &	ROOF FRAM	IING
WIND DDECOURE ZONE	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA	EFF. AREA
WIND PRESSURE ZONE	≤ 10 SF	20 SF	50 SF	100 SF	≥ 200 SF
ROOF ZONE ①	-26.4 PSF	-26.4 PSF	-26.4 PSF	-26.4 PSF	-22.7 PSF
ROOF ZONE ①	-45.9 PSF	-42.9 PSF	-38.9 PSF	-35.9 PSF	-32.8 PSF
ROOF ZONE ②	-60.6 PSF	-56.7 PSF	-51.5 PSF	-47.6 PSF	-43.7 PSF
ROOF ZONE ③	-82.5 PSF	-74.8 PSF	-64.5 PSF	-56.7 PSF	-48.9 PSF

NOTES:

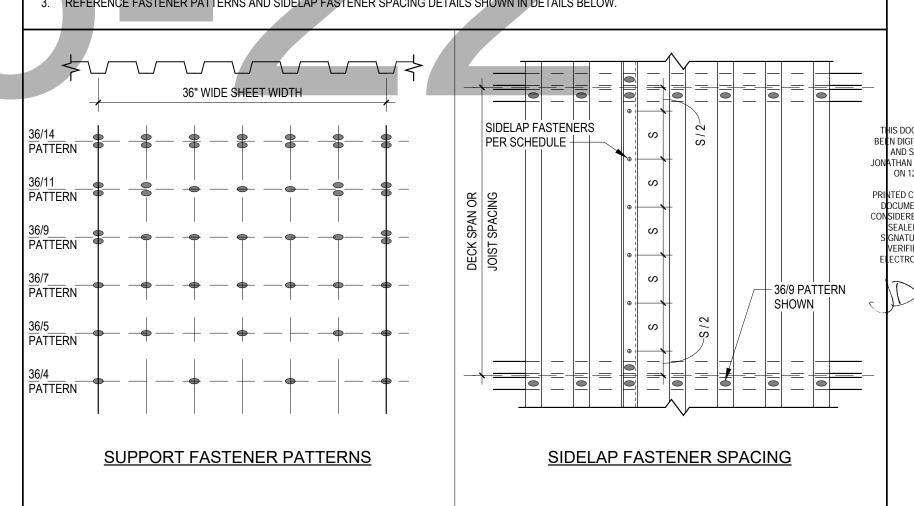
- 1. WIND PRESSURES IN THE TABLES ABOVE ARE BASED ON CALCULATIONS FROM ASCE 7-16.
 2. OVERHANG PRESSURES IN THE TABLES ABOVE SHALL APPLY TO ALL ROOFS OVER BALCONIES, BREEZEWAYS, AND COVERED ENTRIES. COORDINATE WITH ARCHITECTURAL DRAWINGS.

 3. PROVIDE IMPACT RESISTANT GLAZING AS REQUIRED FOR WIND BORNE DERRIS PER FLORIDA BUILDING CODE.
- PROVIDE IMPACT RESISTANT GLAZING AS REQUIRED FOR WIND BORNE DEBRIS PER FLORIDA BUILDING CODE.
 MAXIMUM ALLOWABLE DEAD LOADS TO BE USED TO RESIST UPLIFT SHALL BE AS FOLLOWS:

 NET UPLIFT = ULTIMATE UPLIFT 10 PSF DEAD LOAD
 NET UPLIFT = ALLOWABLE UPLIFT 6 PSF DEAD LOAD

		ROOF DECK	SCHEDULE		
ZONE	DECK TYPE	SUPPORT FASTENERS	FASTENER PATTERN	SIDELAP FASTENERS	FASTENER PER SPAN
D1	1-1/2" - 22 GA - TYPE "B"	5/8" DIA. PUDDLE WELDS	36/7	#12 -14 TEK SCREWS	12 - 6" O.C.
D2	1-1/2" - 22 GA - TYPE "B"	5/8" DIA. PUDDLE WELDS	36/5	#12 -14 TEK SCREWS	6 - 12" O.C.
D3	1-1/2" - 22 GA - TYPE "B"	5/8" DIA. PUDDLE WELDS	36/3	#12 -14 TEK SCREWS	6 - 12" O.C.

1. FASTEN DECK EDGE SIDELAP AT PERIMETER WITH 5/8" DIA. PUDDLE WELDS AT 6" O.C.
2. FASTEN DECK EDGE SUPPORT AT PERIMETER WITH 5/8" DIA. PUDDLE WELDS TO MATCH FASTENER PATTERN AS NOTED IN SCHEDULE.
3. REFERENCE FASTENER PATTERNS AND SIDELAP FASTENER SPACING DETAILS SHOWN IN DETAILS BELOW.

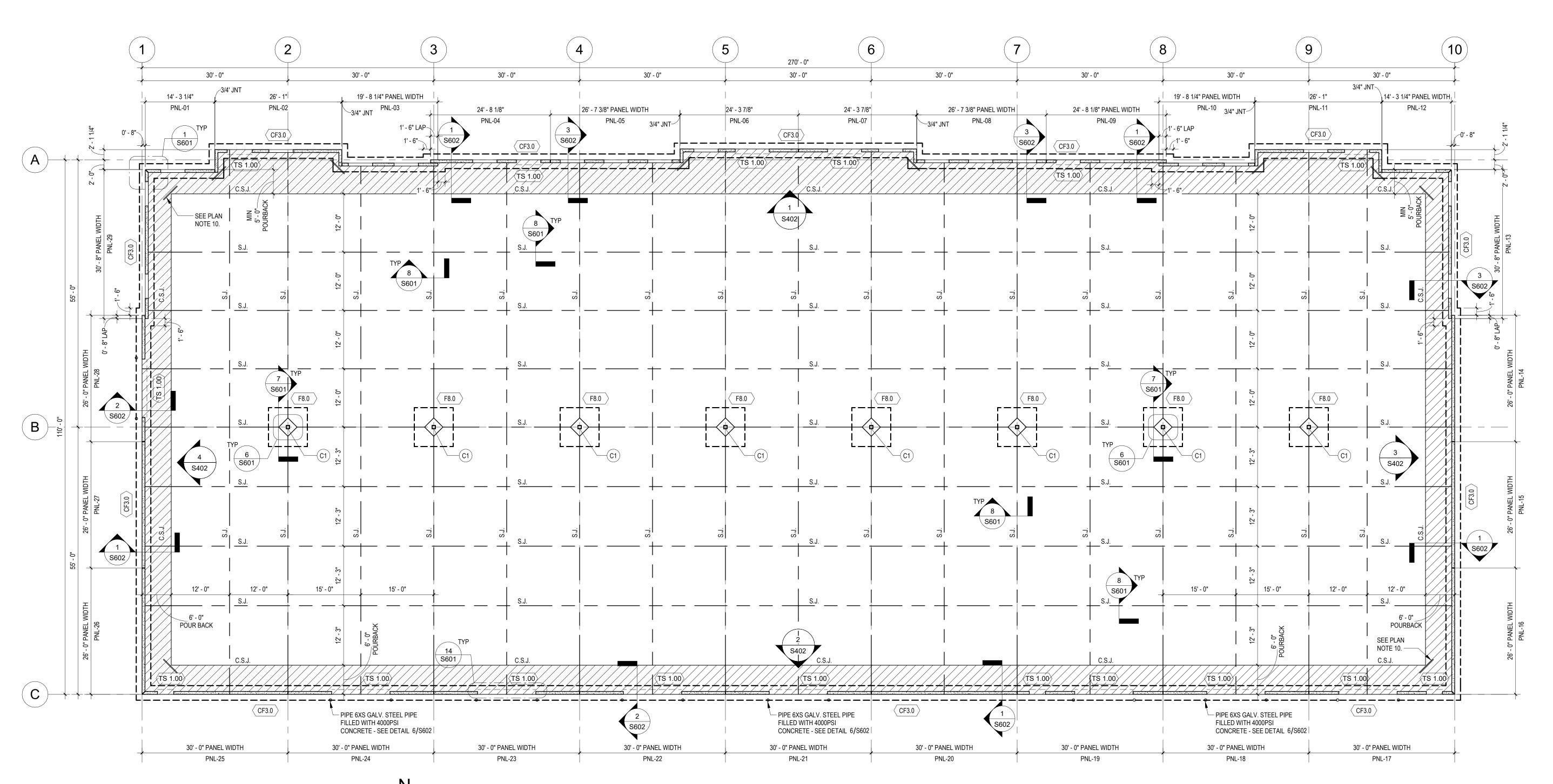


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Job Number:



GENERAL CONTRACTORS, INC.

FOUNDATION AND SLAB ON GRADE PLAN

FOUNDATION PLAN NOTES:

- REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.
- 2. CONTRACTOR TO VERIFY ALL ELEVATIONS AND DIMENSIONS SHOWN WITH ARCHITECTURAL DRAWINGS AND EQUIPMENT SUPPLIERS SHOP DRAWINGS PRIOR TO FABRICATION AND / OR START OF CONSTRUCTION.
- 3. COORDINATE EXISTING / INSTALLED UNDERGROUND UTILITIES AND OTHER BURIED PIPES AND CONDUITS PRIOR TO PLACEMENT OF FOOTINGS. DO NOT PLACE BUILDING FOUNDATIONS OVER EXISTING / INSTALLED PIPES AND CONDUITS UNLESS APPROVED OTHERWISE.
- 4. T.O. FOOTING ELEVATION IS AT -1'-0" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION DETAIL SHEETS AND SCHEDULES FOR FOUNDATION SIZE AND REINFORCEMENT.
- 5. EXTEND ALL CONTINUOUS FOOTING REINFOREMENT INTO ADJACENT SPREAD FOOTINGS A
- 6. ALL WALLS AND COLUMNS ARE TO BE CENTERED ON FOUNDATIONS UNLESS NOTED

MINIMUM DISTANCE OF 4'-0".

- OTHERWISE. SEE PLAN DIMENSIONS FOR OFFSETS.
- 7. PREPARE THE SLAB SUB-BASE AND COMPACT THE SOIL PER THE PROJECT GEOTECHNICAL REPORT, THE CIVIL DRAWINGS, AND THE STRUCTURAL GENERAL NOTES. IF ANY OF THESE DRAWINGS OR NOTES ARE IN CONFLICT, THE CONTRACTOR MUST ALERT THE ENGINEER FOR CLARIFICATION PRIOR TO START OF CONSTRUCTION.
- 8. T.O. SLAB ELEVATION IS AT 0'-0" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION AND SLAB ON GRADE DETAIL SHEETS.
- 9. REFERENCE THE CONCRETE SLAB ON GROUND SCHEDULE, INCLUDED ON THIS SHEET, FOR FLOOR SLAB THICKNESS AND REINFORCEMENT.
- 10. PROVIDE (2) #4 x 4'-6" LG BARS AT TOP OF SLAB AT ALL RE-ENTRANT CORNERS AND DISCONTINUOUS ENDS OF SLAB SAW-CUT JOINTS.
- 11. REFERENCE THE ARCHITECTURAL DRAWINGS FOR SLAB EDGES, FLOOR SLOPES, WALL OPENINGS, AND OTHER DIMENSIONS NOT GIVEN. CONTRACTOR MUST COORDINATE AND VERIFY ALL DIMENSIONS WITH PROJECT ARCHITECT PRIOR TO FABRICATION.

<u>LEGEND</u>

EL = #' - ##"

T.O. FTG

INDICATES TOP OF CONCRETE FOOTING ELEVATION.

F#.# INDICATES SPREAD FOOTING TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.

CF#.# INDICATES CONTINUOUS FOOTING TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.

INDICATES THICKENED SLAB EDGE TYPE. SEE FOUNDATION

SCHEDULE FOR SIZE AND REINFORCEMENT.

INDICATES STEEL COLUMN TYPE. SEE STEEL COLUMN SCHEDULE FOR COLUMN SIZE AND BASEPLATE DETAILS.

INDICATES CONCRETE COLUMN TYPE. SEE CONCRETE COLUMN SCHEDULE FOR COLUMN SIZE AND REINFORCEMENT DETAILS.

S.F. INDICATES STEPPED FOOTING PER DETAILS 2/S601 AND 3/S601.S.J. INDICATES SLAB SAWCUT JOINT PER DETAIL 1/S602.

C.S.J. INDICATES CONSTRUCTION JOINT PER TYPICAL DETAIL 2/S602 OR AS REQUIRED PER CONSTRUCTION SEQUENCING.

	SLAB ON GRADE SCHEDULE	
<u>PLAN MARK</u>	SLAB CONSTRUCTION	<u>REMARKS</u>
A	6" THICK CONCRETE SLAB ON GRADE REINF w/ 6x6 - W1.4xW1.4 WELDED WIRE MESH. PROVIDE 15 MIL VAPOR RETARDER ON TERMITE TREATED COMPACTED SUBGRADE. PROVIDE (3) #3 BAR AT 18" O.C. ALONG PERIMETER OF TYPICAL SLAB ADJACENT TO POURBACK JOINT.	TYPICAL SLAB
В	SLAB ON GRADE POURBACK STRIP. THICKNESS AND TYPICAL REINFORCEMENT TO MATCH TYPICAL SLAB. PROVIDE ADDITIONAL #4 AT 24" O.C. IN THE SHORT DIRECTION AND #4 AT 48" O.C. IN THE LONG DIRECTION. REFERENCE TYPICAL DETAILS AND SECTIONS FOR REQUIRED REINFORCEMENT AT SLAB JOINTS.	TYPICAL POURBACK
C	8" THICK CONCRETE SLAB ON GRADE REINF w/ #4 BARS AT 12" O.C. EACH WAY, TOP AND BOTTOM. SEE GENERAL NOTES FOR CONCRETE COVER TO REINFORCEMENT REQUIREMENTS.	DUMPSTER PAD SEE 1/S603.



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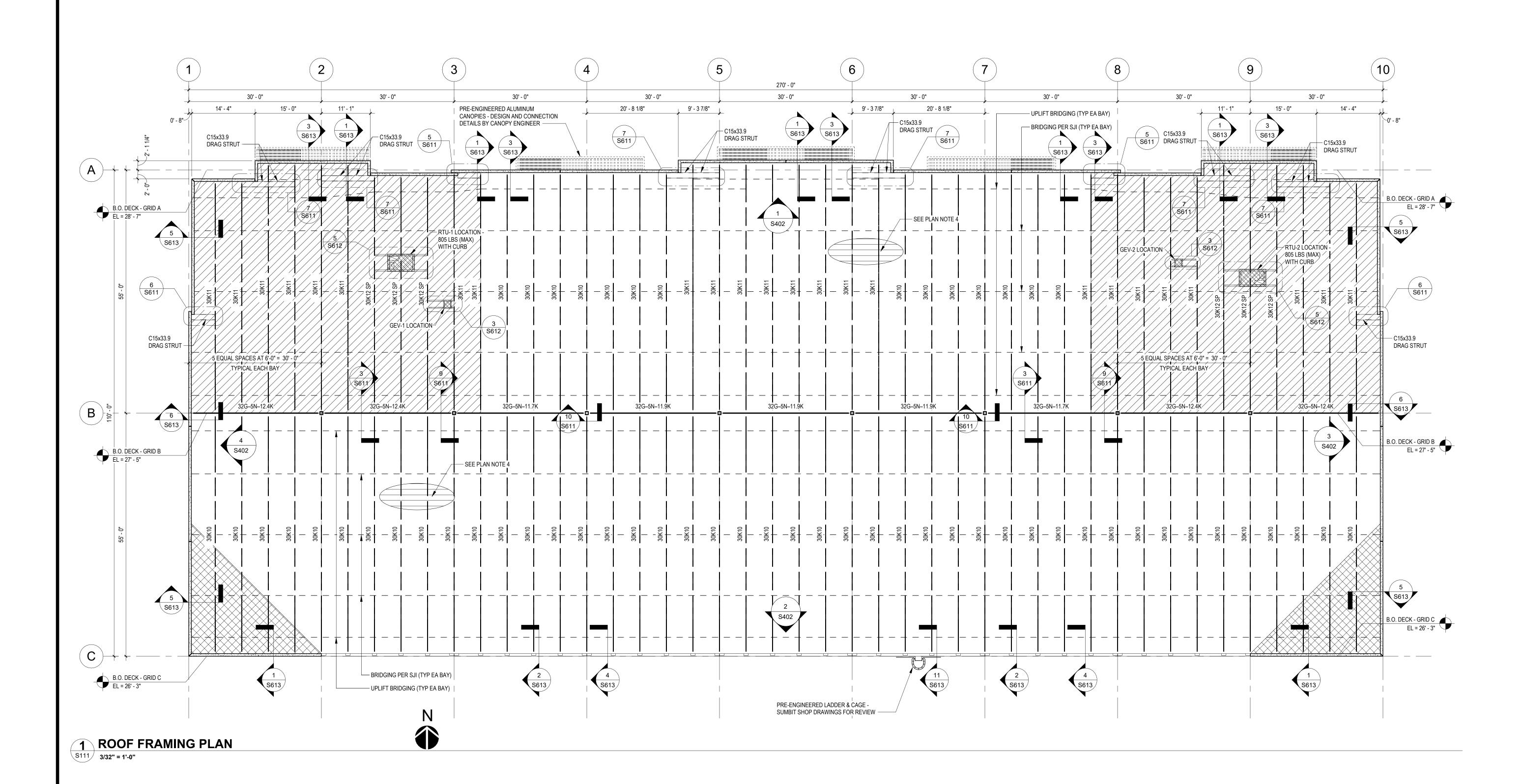
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ROOF FRAMING PLAN NOTES:

1. REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.

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- 2. TOP OF STEEL ELEVATIONS MAY VARY. SEE THE STRUCTURAL AND ARCHITECTURAL PLANS AND SECTIONS FOR REQUIRED ELEVATIONS. (#-##") INDICATES TOP OF STEEL ELEVATION.
- 3. CONTRACTOR SHALL VERIFY ALL ELEVATIONS AND DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AND EQUIPMENT SUPPLIERS SHOP DRAWINGS PRIOR TO FABRICATION AND/OR START OF CONSTRUCTION.
- 4. ROOF DECK SHALL BE GALV 1-1/2" 22GA WIDE RIB METAL DECK, (U.N.O.) SPANNING OVER OPEN-WEB STEEL JOISTS SPACED AT 6'-0" O.C. (MAX) OR AS INDICATED ON ROOF FRAMING
 - DECK SECTION PROPERTIES: $lp = 0.155 ext{ in}^4 / \text{ft}$ $ln = 0.183 ext{ in}^4 / \text{ft}$
 - $Sp = 0.186 in^3 / ft$ $Sn = 0.192 in^3 / ft$ Fy = 80 KSI
- 5. FASTEN ROOF DECK TO ALL SUPPORTS w/ 5/8" DIA. PUDDLE WELDS AND ATTACH DECK SIDELAPS w/ #12 TEK SCREWS. REFERENCE THE ROOF DECK FASTENING SCHEDULE AND ROOF DECKING PLAN PROVIDED ON SHEET S002.
- PROVIDE L4x4x3/8 EDGE ANGLE AROUND ALL OPENINGS AND AROUND PERIMETER OF ROOF. COORDINATE WITH ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF EDGE ANGLES. SEE DECK ANGLE SPLICE DETAIL 4/S611
- (5" L), (5" R), OR (5" L/R) INDICATE 5" JOIST SEAT AT END OF STEEL JOIST AS NOTED IN LIEU OF STANDARD 2-1/2" SEAT. STEEL DETAILER SHALL COORDINATE T.O. STEEL ELEVATIONS WITH JOIST MANUFACTURER.
- 8. OPEN WEB STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE SPECIFICATION OF THE STEEL JOIST INSTITUTE. PROVIDE JOIST REINFORCEMENT PER TYPICAL DETAIL 1/S611AT CONCENTRATED LOADS > 150 LBS.
- PROVIDE STANDARD JOIST BRIDGING AND UPLIFT BRIDGING PER LATEST SJI SPECIFICATIONS AND THE STEEL JOIST SHOP DRAWINGS (TYPICAL).
- 10. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS AND DETAILS NOT SHOWN. RESOLVE ALL DISCREPANCIES PRIOR TO FABRICATION.
- 11. COORDINATE ALL ROOF OPENINGS AND MISCELLANEOUS ROOF DECK PENETRATIONS AND EQUIPMENT WITH THE ARCHITECTURAL AND MEP DRAWINGS. REFERENCE THE TYPICAL ROOF OPENING DETAILS ON SHEET S612.
- 12. JOISTS NOTED AS "30K12 SP" SHALL BE DESIGNED FOR PROJECT TYPICAL UNIFORM LOADING AS WELL AS SERVICE CONCENTRATED LOADS AS NOTED AT THE CURB FRAMES:
 DEAD LOAD: 252 LBS

<u>LEGEND</u>

(#' - ##") INDICATES BOTTOM OF DECK ELEVATION. SEE PLAN.

WIND LOAD: + / - 540 LBS

- c = #" INDICATES REQUIRED POSITIVE CAMBER IN W-BEAM.
- INDICATES RIGID MOMENT CONNECTION AT SUPPORT.

 INDICATES KNEE-BRACE AT BEAM-COLUMN JOINT. SEE STEEL
- K.B. ELEVATIONS ON S301 FOR DETAILS.
- D.B. INDICATES FULL- OR HALF-DEPTH DIAGONAL BRACE. SEE STEEL ELEVATIONS ON \$301 FOR DETAILS.
- INDICATES STEEL COLUMN TYPE. SEE STEEL COLUMN SCHEDULE FOR COLUMN SIZE AND BASEPLATE DETAILS.
- INDICATES CONCRETE COLUMN TYPE. SEE CONCRETE COLUMN SCHEDULE FOR COLUMN SIZE AND REINFORCEMENT DETAILS.
- R.D. SINDICATES ROOF DRAIN LOCATIONS. REFER TO ARCHITECTURAL AND MEP DRAWINGS FOR COORDINATION.
- REGION INDICATES "OFFICE" LOADING. REFERENCE TABLE 1 ON SHEET S002 FOR DESIGN LOAD DATA.
- REGION INDICATES ADDITIONAL TAPERED INSULATION REQUIRED TO CREATE ALTERNATE ROOF SLOPE.



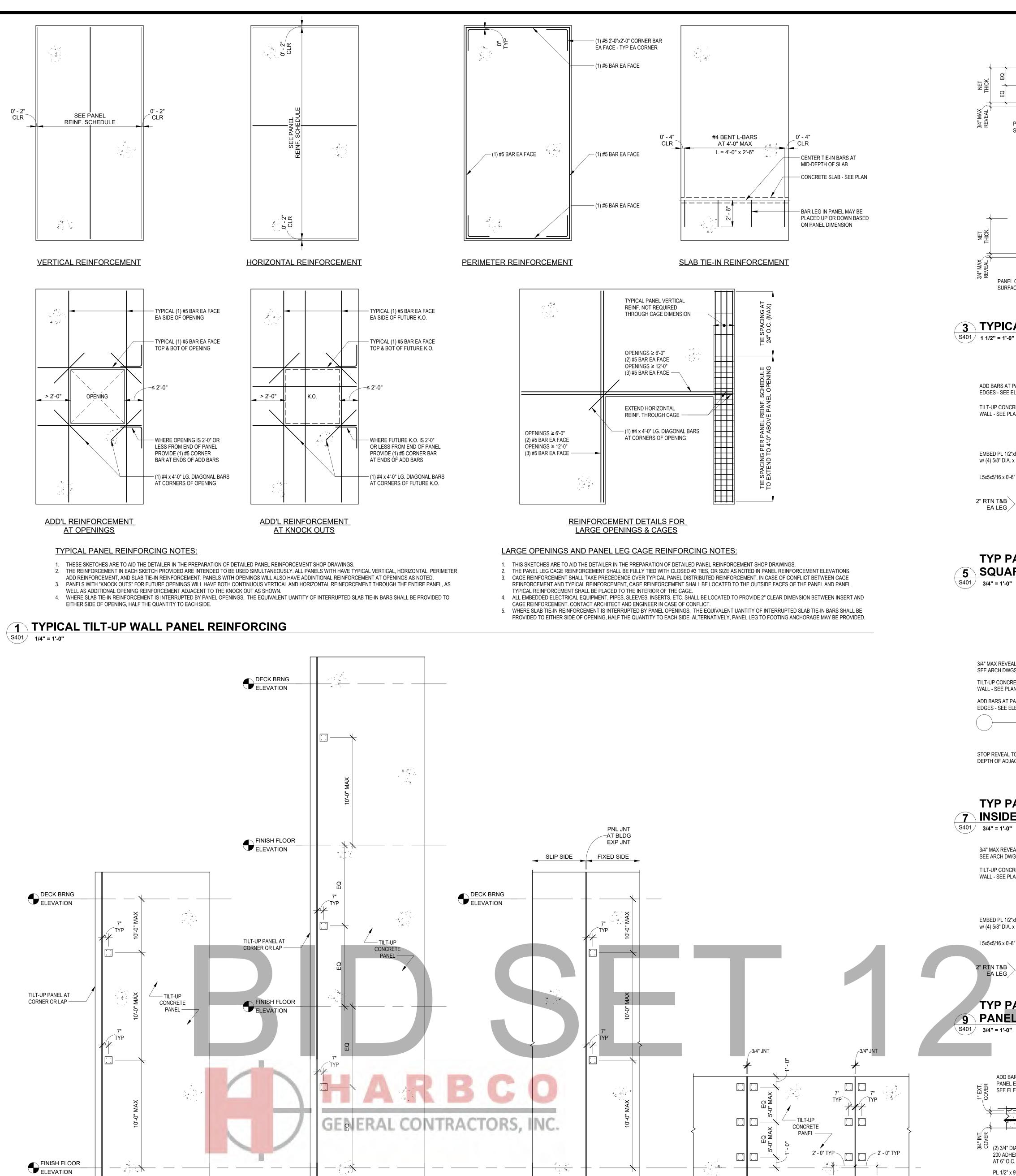
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A/E Job Number:

Job Number: **18406**



EXPANSION PANEL JOINT

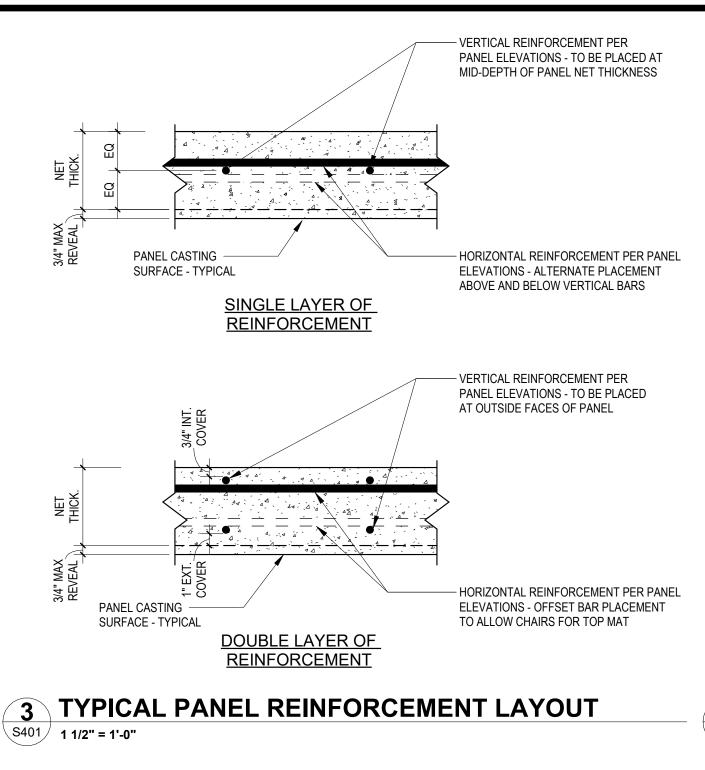
TYPICAL CANTILEVER PANELS

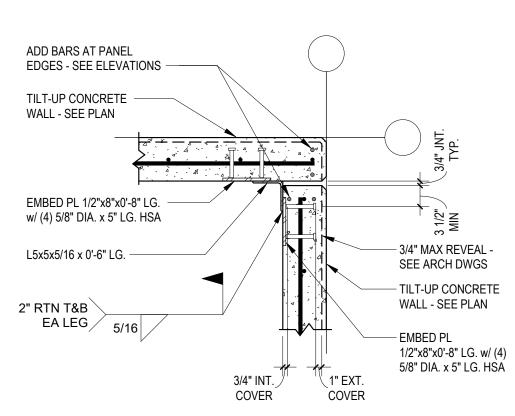
TYPICAL MULTI-STORY PANEL

TYPICAL SINGLE STORY PANEL

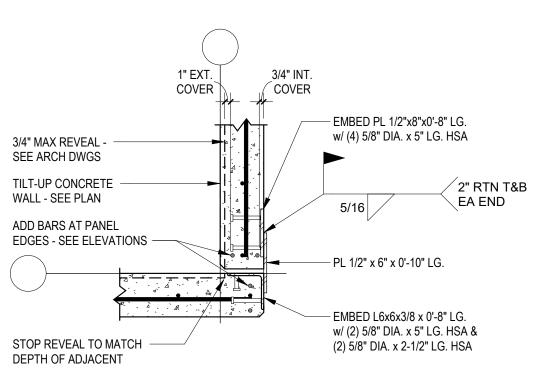
S401 1/4" = 1'-0"

2 TYPICAL TILT-UP WALL PANEL-TO-PANEL CONNECTION EMBED LAYOUT

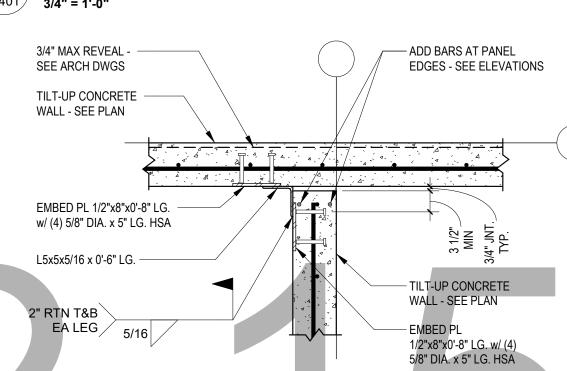




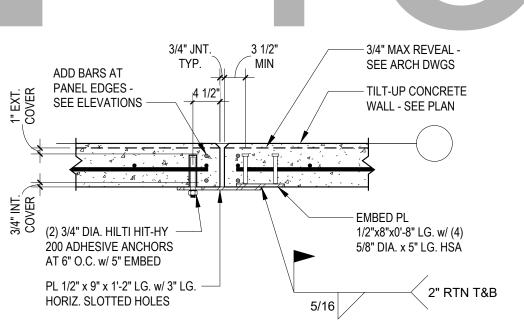
TYP PANEL TO PANEL CONNECTION **5** SQUARE OR BUTT JOINT



TYP PANEL TO PANEL CONNECTION 7 INSIDE CORNER JOINT

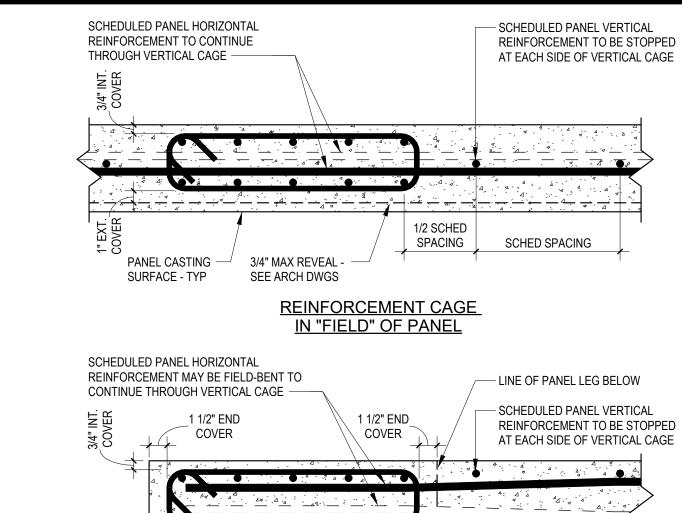


TYP PANEL TO PANEL CONNECTION PANEL "T" JOINT



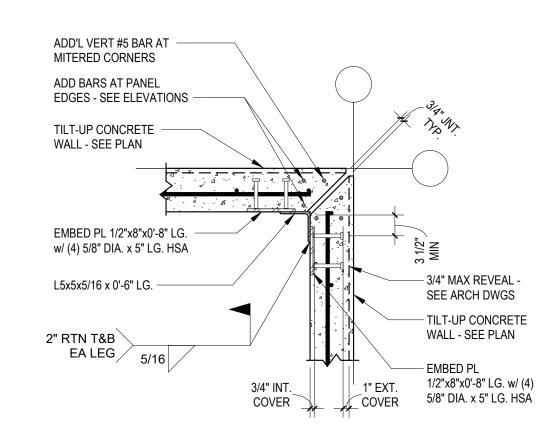
TYP PANEL TO PANEL CONNECTION **11 AT BLDG EXPANSION JOINT**

S401 3/4" = 1'-0"



REINFORCEMENT CAGE <u>IN "LEG" OF PANEL</u> 4 TYPICAL PANEL CAGE REINFORCEMENT

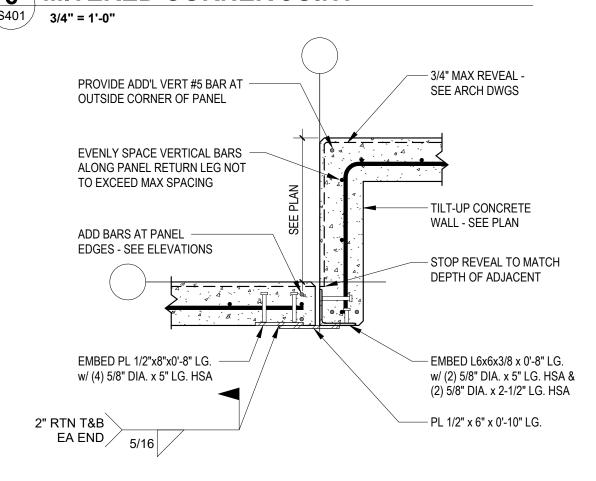
SPACING



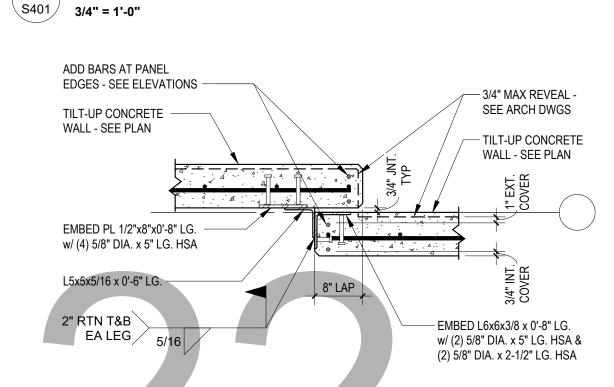
PANEL CASTING — 3/4" MAX REVEAL -

SURFACE - TYP

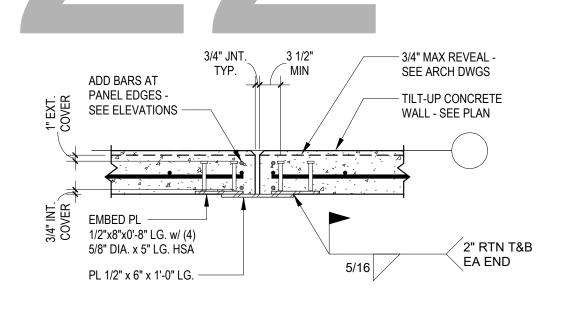
TYP PANEL TO PANEL CONNECTION **6** MITERED CORNER JOINT



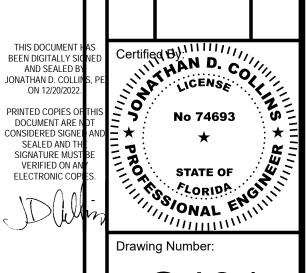
TYP PANEL TO PANEL CONNECTION **8 PANEL RETURN JOINT**



TYP PANEL TO PANEL CONNECTION 10 PANEL LAP JOINT



TYP PANEL TO PANEL CONNECTION 12 AT CANTILEVER PANELS S401 3/4" = 1'-0"



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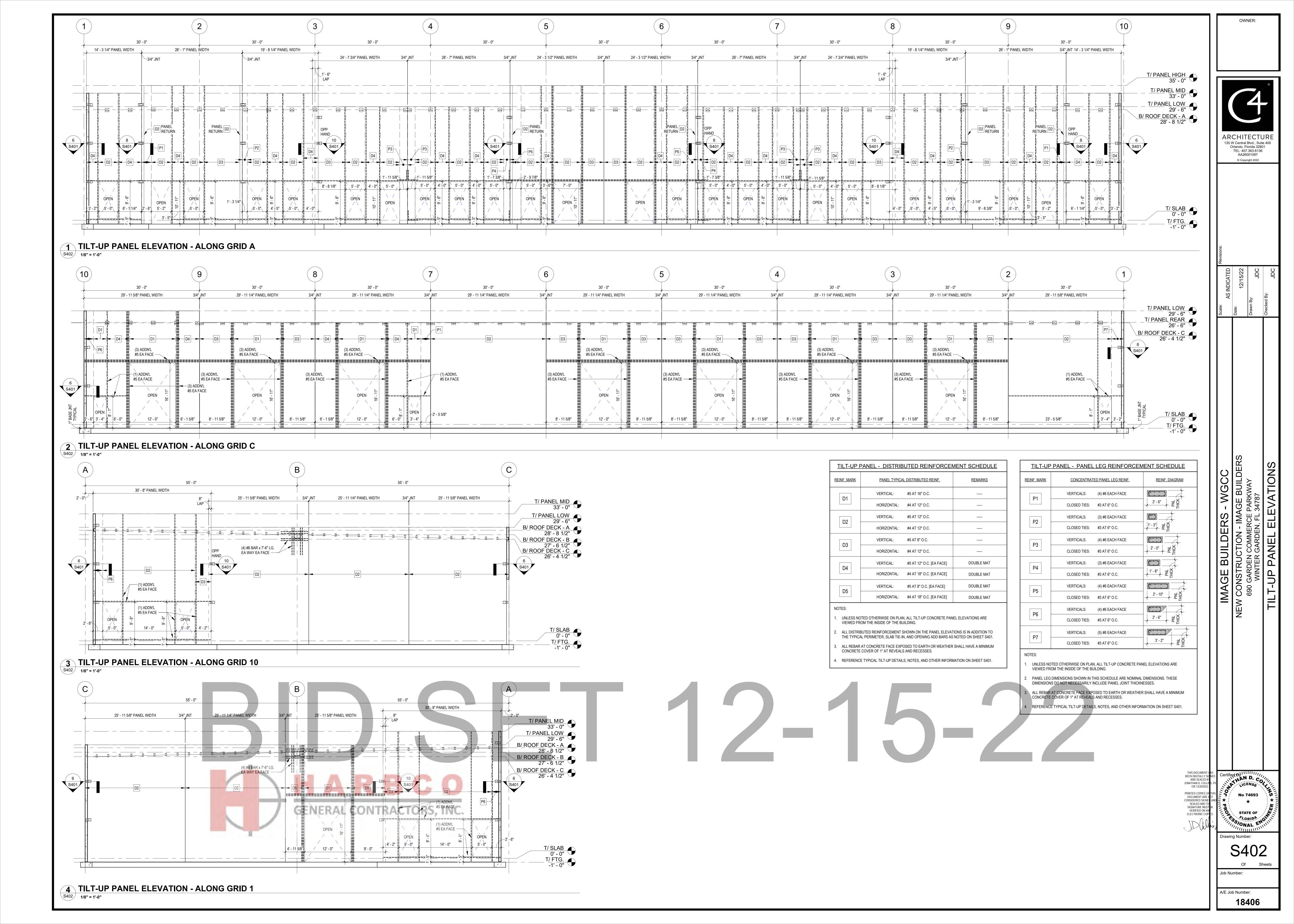
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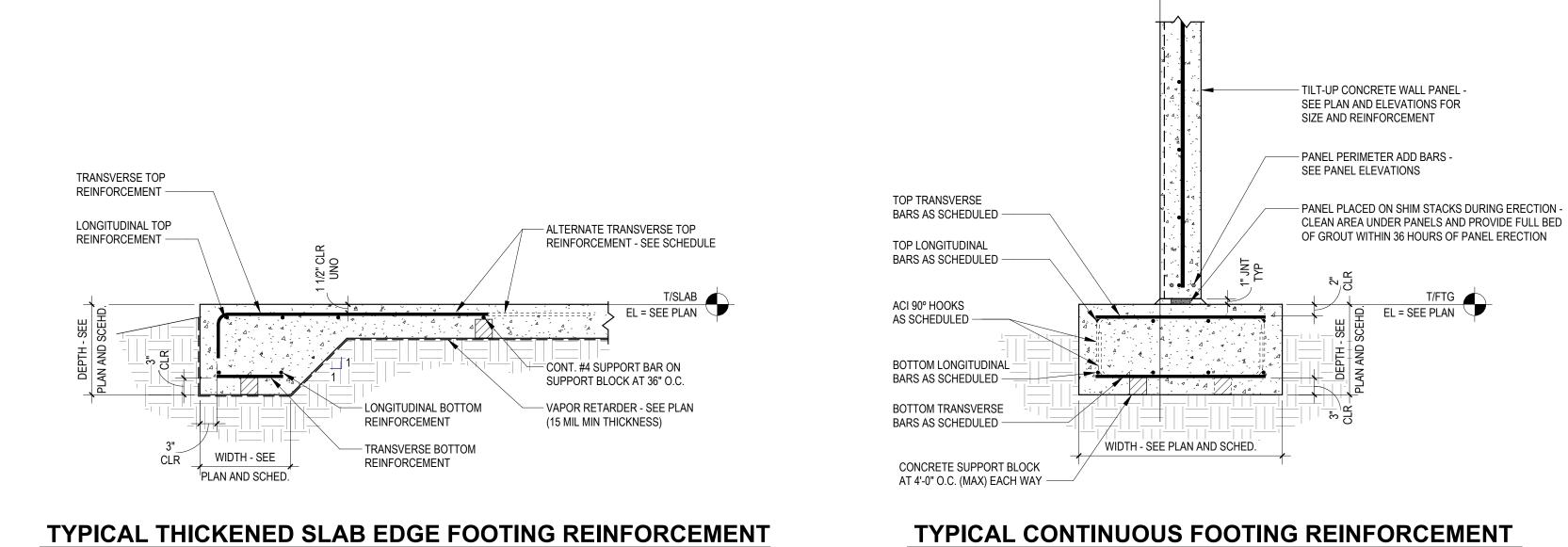
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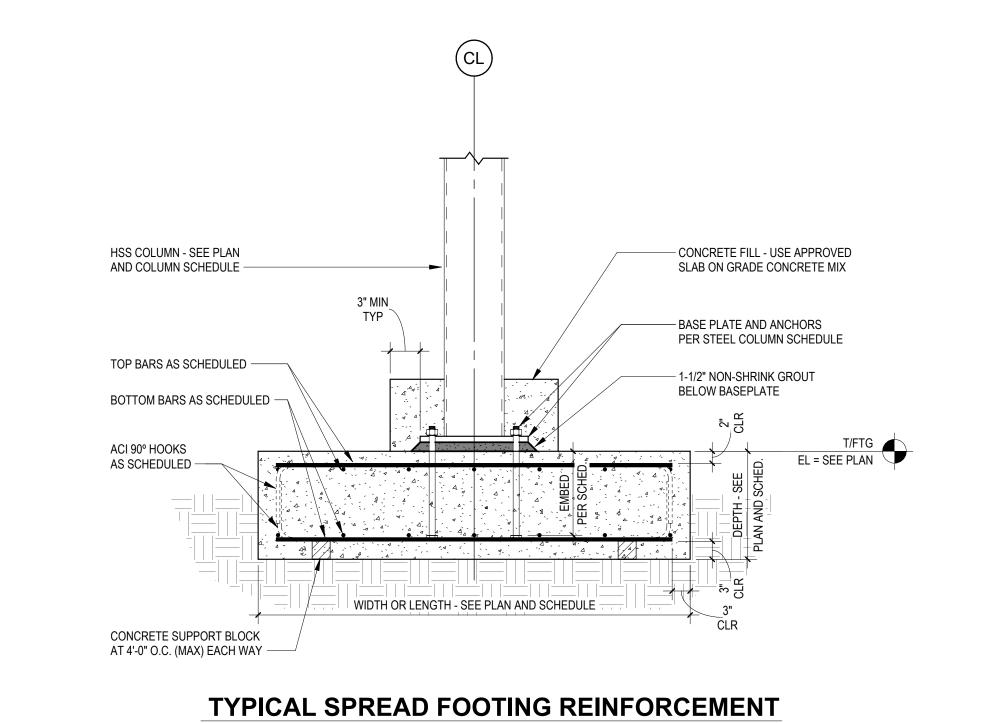
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S401 Of Sheets Job Number:

18406







		<u>C(</u>	<u> DNCRE</u>	TE FOOTING SCHEDULE	
TYPE	SIZE			REINFORCING	REMARKS
111 6	WIDTH	LENGTH	DEPTH	KLINI OKOINO	KLIVIAKKO
(F 8.0)	8' - 0"	8' - 0"	1' - 6"	(9) #6 x 7' - 6" LG. EA WAY TOP	SPREAD FOOTI
1 0.0	0 - 0	0 - 0	1 - 0	(9) #6 x 7' - 6" LG. EA WAY BOTTOM	SFILAD I OOTI
(CE 2.0)	3' - 0"	00NT 41 01		LONG. (4) #5 x CONT. BOTTOM	CONTINUOUS FO
(CF 3.0)	3-0	CONT.	1' - 0"	TRANS. #4 x 2'-6" LG. AT 16" O.C. BOTTOM	CONTINUOUS FOOTIN
(CF 4.5)	4' - 6"	4' - 6" CONT.	1' - 4"	LONG. (5) #5 x CONT. TOP & BOTTOM	CONTINUOUS FOOT
CF 4.5	4 - 0	CONT.	1 - 4	TRANS. #4 x 4'-0" LG. AT 12" O.C. TOP & BOTTOM	CONTINUOUS FOC
(TO 1 00)	44 00	001/7	41 011	LONG. (2) #4 x CONT. TOP LONG. (2) #4 x CONT. BOTTOM	TURKENED OLAD
(TS 1.00)	1' - 0"	CONT.	1' - 0"	ALT. TRANS. #4 x 3'-6" & 5'-0" LG. w/ ACI 90° HOOK TOP #4 x 1'-0" LG. SKEWED SUPPORT BAR AT 36" O.C. BOT	THICKENED SLAB EDO
(TO 1 OO)	41 41	2217	41 011	LONG. (2) #5 x CONT. TOP LONG. (2) #5 x CONT. BOTTOM	TURKENED CO.
(TS 1.33)	1' - 4"	CONT.	1' - 0"	ALT. TRANS. #4 x 4'-0" & 5'-6" LG. w/ ACI 90° HOOK TOP #4 x 1'-0" LG. SKEWED SUPPORT BAR AT 36" O.C. BOT	THICKENED SLAB

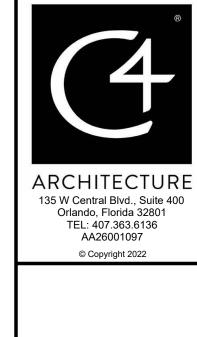
TENSION DEVELOPMENT LENGTHS AND LAP SPLICE LENGTHS FOR UNCOATED BARS																									
3000 PSI 4000 PSI 5000 PSI 6000 PSI					7000 PSI				≥ 8000 PSI																
BAR SIZE	LAP CLASS	TOP	BARS	OTHER	R BARS	TOP	BARS	OTHER	R BARS	TOP	BARS	OTHE	R BARS	TOP BARS OTHER BARS		TOP BARS OTHER		R BARS	S TOP BARS		OTHER BARS				
O.L.L	02/100	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2	CASE 1	CASE 2
# 3	Α	22	32	17	25	19	28	15	22	17	25	13	19	15	23	12	18	14	21	12	16	13	20	12	15
# 3	В	28	42	22	32	24	36	19	28	22	33	17	25	20	30	15	23	18	28	14	21	17	26	13	20
# 4	Α	29	43	22	33	25	37	19	29	22	33	17	26	20	31	16	24	19	28	15	22	18	26	14	20
# 4	В	37	56	29	43	32	48	25	37	29	43	22	33	26	40	20	31	25	37	19	28	23	34	18	26
# 5	Α	36	54	28	41	31	47	24	36	28	42	22	32	25	38	20	29	24	35	18	27	22	33	17	25
# 3	В	47	70	36	54	40	60	31	47	36	54	28	42	33	49	25	38	31	46	24	35	29	43	22	33
#6	Α	43	64	33	50	37	56	29	43	33	50	26	38	31	46	24	35	28	42	22	33	26	40	20	30
# 0	В	56	84	43	64	48	72	37	56	43	65	33	50	40	59	31	46	37	55	28	42	40	51	26	40
#7	Α	63	94	48	72	54	81	42	63	49	73	37	56	44	66	34	51	41	61	32	47	38	58	30	44
# /	В	81	122	63	94	70	106	54	81	63	94	49	73	58	86	44	66	53	80	41	61	50	75	38	58
#8	Α	72	107	55	82	62	93	48	71	55	63	43	64	51	76	39	58	47	70	36	54	44	66	34	51
# 0	В	93	139	72	107	80	121	62	93	72	108	55	83	66	98	51	76	61	91	47	70	57	85	44	66
# 9	Α	81	121	62	93	70	105	54	81	63	94	48	72	57	85	44	66	53	79	41	61	49	74	38	57
π 3	В	105	157	81	121	91	136	70	105	81	122	63	94	74	111	57	85	69	103	53	79	64	96	49	74
# 10	Α	91	136	70	105	79	118	61	91	70	105	54	81	64	96	49	74	59	89	46	69	56	83	43	64
# 10	В	118	177	91	136	102	153	79	118	91	137	70	105	83	125	64	96	77	116	59	89	72	108	56	83
# 11	Α	101	151	76	116	87	131	67	101	76	117	60	90	71	107	55	82	66	99	51	76	62	93	48	71
# 11	В	131	196	101	151	113	170	87	131	101	152	78	117	93	139	71	107	86	128	66	99	80	120	62	93
# 14	N/A ^[5]	121	161	93	139	105	157	81	121	94	140	72	108	86	128	66	99	79	119	61	91	74	111	57	85
# 18	N/A ^[5]	161	241	124	186	139	209	107	161	125	187	96	144	114	171	88	131	106	158	81	122	99	148	76	114

- 1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE. ALL VALUES ARE LENGTHS IN INCHES.
- 2. TENSION DEVELOPMENT AND LAP SPLICE LENGTHS ARE CALCULATED PER ACI 318-14, SECTION 25.4.2 AND 25.5.2, RESPECTIVELY. TABULATED VALUES FOR BEAMS AND COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS.
- 3. CASE 1 AND CASE 2 ARE DEPENDENT ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND THE CENTER-TO-CENTER SPACING OF THE BARS. CASES ARE DEFINED AS FOLLOWS:
- A. BEAMS AND COLUMNS:
- a. CASE 1: COVER OF AT LEAST 1.0 (db) AND C-C SPACING OF AT LEAST 2.0 (db) WHERE (db) IS THE DIAMETER OF THE BAR.
 b. CASE 2: COVER LESS THAN 1.0 (db) OR C-C SPACING LESS THAN 2.0 (db)
- B. ALL OTHER STRUCTURAL MEMBERS:
 a. CASE 1: COVER OF AT LEAST 1.0 (db) <u>AND</u> C-C SPACING OF AT LEAST 3.0 (db)
 b. CASE 2: COVER LESS THAN 1.0 (db) <u>OR</u> C-C SPACING LESS THAN 3.0 (db)
- 4. LAP SPLICE LENGTHS ARE MULTIPLES OF THE CALCULATED TENSION DEVELOPMENT LENGTH PER ACI 318-14, SECTION 25.2.2 AS FOLLOWS:
 A. CLASS "A" = 1.0 (Ld)
- B. CLASS "B" = 1.3 (Ld)
- 5. ACI 318-14 DOES NOT ALLOW TENSION LAP SPLICES OF #14 OR #18 BARS. THE TABULATED VALUES FOR THESE BAR SIZES ARE THE TENSION DEVELOPMENT LENGTHS.
- 6. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.
- 7. FOR LIGHTWEIGHT AGGREGATE CONCRETE, ALL TABULATED VALUES SHALL BE MULTIPLIED BY A FACTOR OF 1.3.

HARE	CO		
GENERAL CONTRA	CTORS, INC.		

	TENSI	ON DEVE HO		LENGTH: UNCOATE		<u>STANDAR</u>	<u>D</u>
BAR SIZE	3000 PSI	3500 PSI	4000 PSI	5000 PSI	6000 PSI	7000 PSI	≥8000 P\$
# 3	10	9	9	8	7	7	6
# 4	13	12	12	10	10	9	8
# 5	17	16	15	13	12	11	10
# 6	20	19	17	16	14	13	12
#7	23	22	20	18	17	15	14
# 8	27	25	23	21	19	18	16
# 9	30	28	26	23	21	20	18
# 10	34	31	29	26	24	22	21
# 11	37	35	32	29	27	25	23
# 14	45	42	39	35	32	29	28
# 18	60	55	52	46	45	39	37

- 1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE. ALL VALUES
- 2. TENSION DEVELOPMENT LENGTHS OF STANDARD HOOKS ARE CALCULATED PER ACI 318-14, SECTION 25.4.3.
- 3. FOR BAR SIZES #3 THROUGH #11 ONLY, THE FOLLOWING FURTHER REDUCTIONS IN LENGTH CAN BE APPLIED:
- A. IF CONCRETE COVER CONFORMS TO ACI 318-14, SECTION 25.4.3.2:
 a. A MODIFICATION FACTOR OF 0.7 MAY BE APPLIED, HOWEVER:
- b. THE FINAL CALCULATED LENGTH OF THE HOOK SHALL NOT BE LESS THAN EITHER 8.0 (db) NOR 6 INCHES.
- B. IF HOOK IS ENCLOSED IN TIES OR STIRRUPS PER ACI 318-14, SECTION 25.4.3.2:
 a. A MODIFICATION FACTOR OF 0.8 MAY BE APPLIED, HOWEVER:
- b. THE FINAL CALCULATED LENGTH OF THE HOOK SHALL NOT BE LESS THAN EITHER 8.0 (db) **NOR** 6 INCHES. 4. FOR LIGHTWEIGHT AGGREGATE CONCRETE, ALL TABULATED VALUES SHALL BE MULTIPLIED BY A FACTOR OF 1.3.

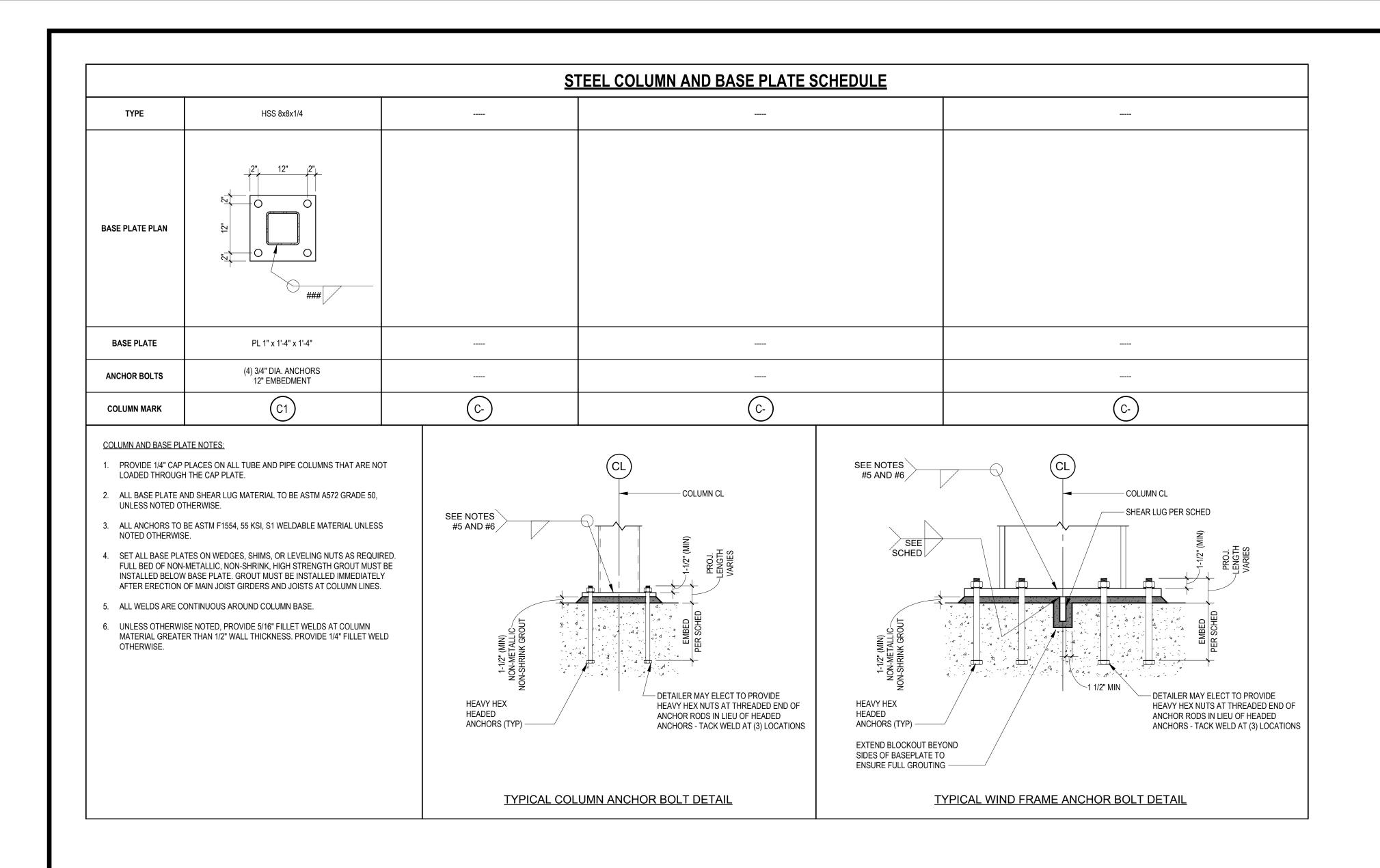


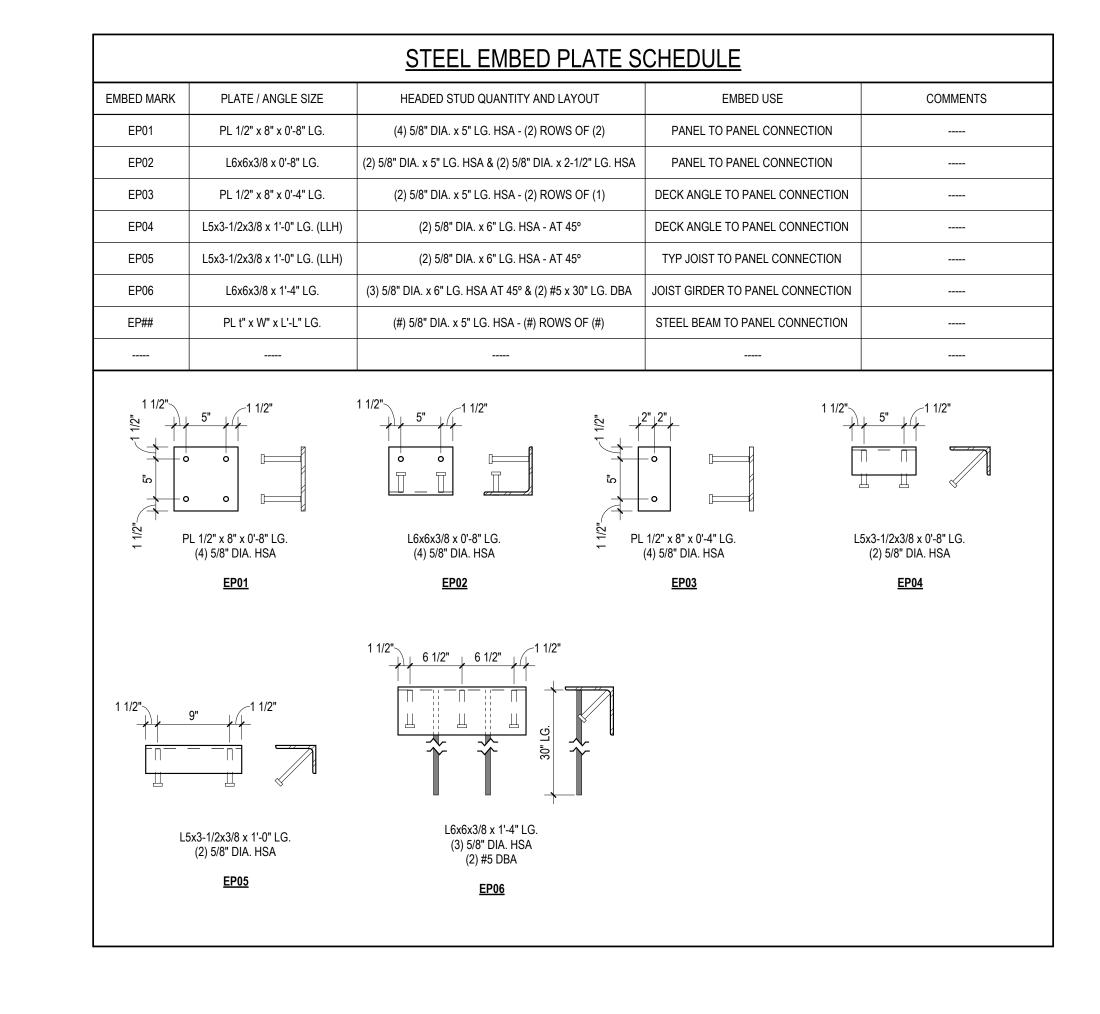
Scale: Revisions: AS INDICATED	Date: 12/15/22	Drawn By: JDC	Checked By: JDC
IMAGE BUILDERS - WGCC	NEW CONSTRUCTION - IMAGE BUILDERS	690 GARDEN COMMERCE PARKWAY WINTER GARDEN, FL 34787	SCHEDULES

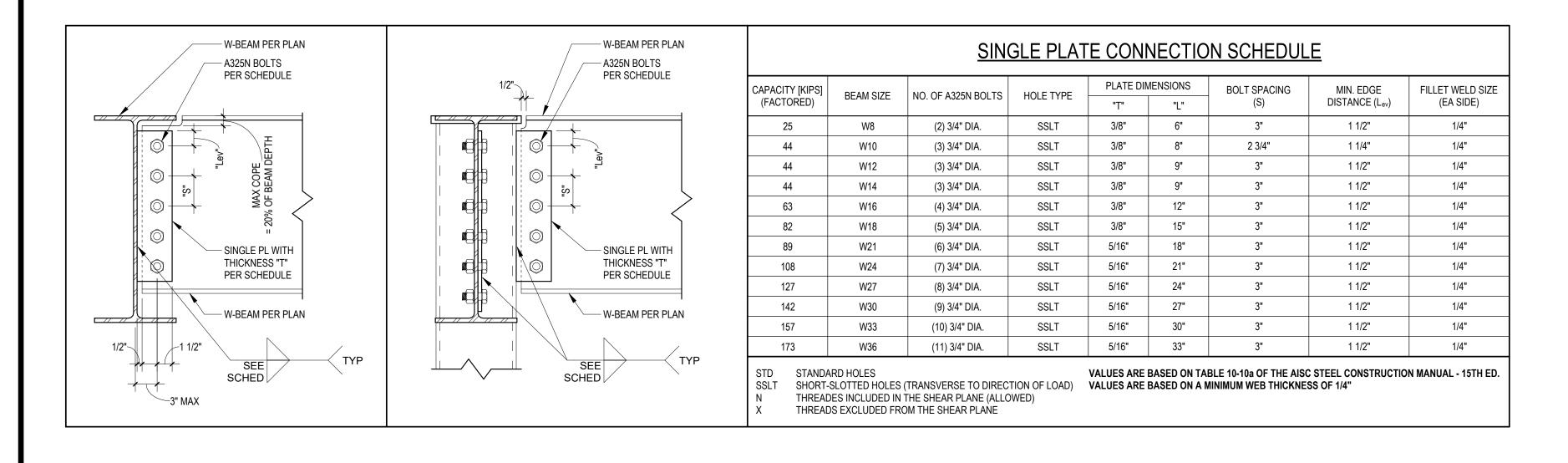
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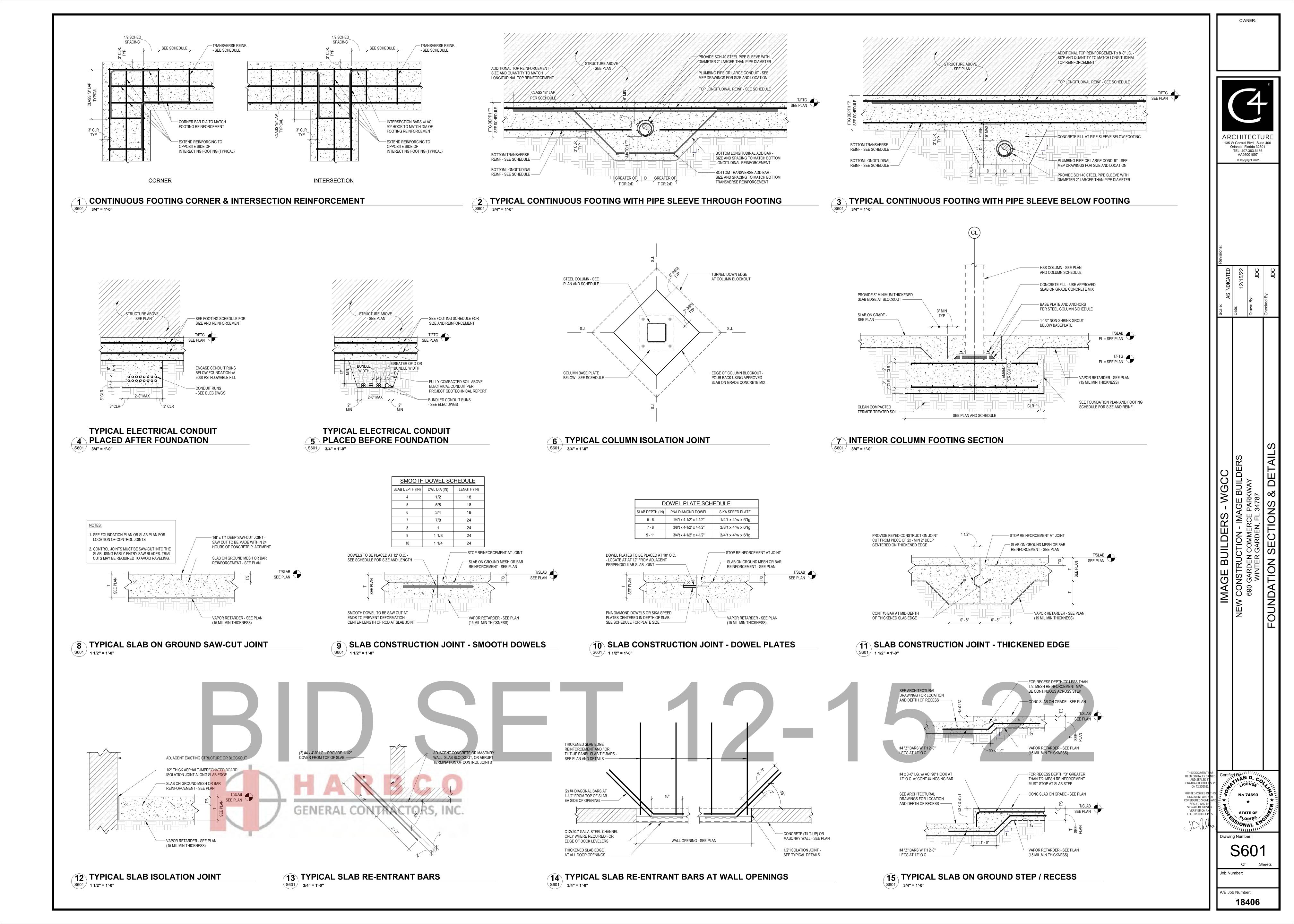
IMAGE BUILDERS - WGCC

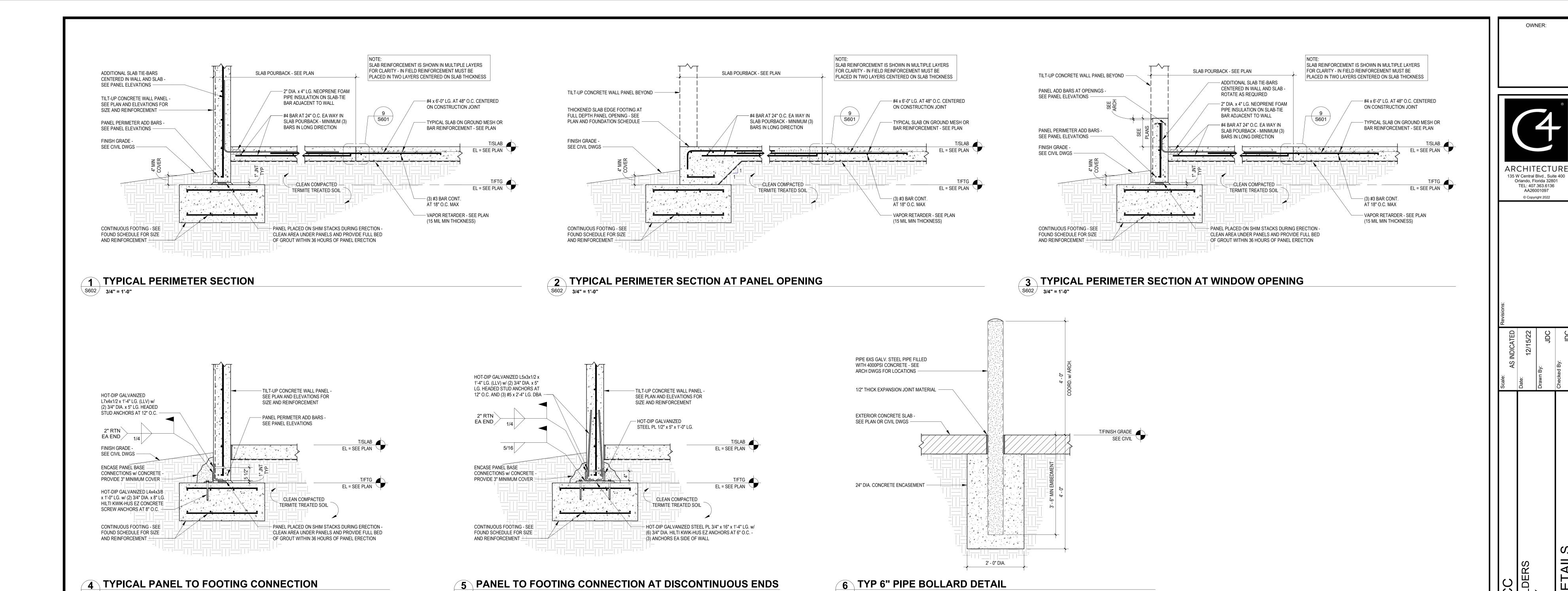
NEW CONSTRUCTION - IMAGE BUILDERS

690 GARDEN COMMERCE PARKWAY
WINTER GARDEN, FL 34787

S502

lob Number:





S602 3/4" = 1'-0"



GENERAL CONTRACTORS, INC.

S602 3/4" = 1'-0"

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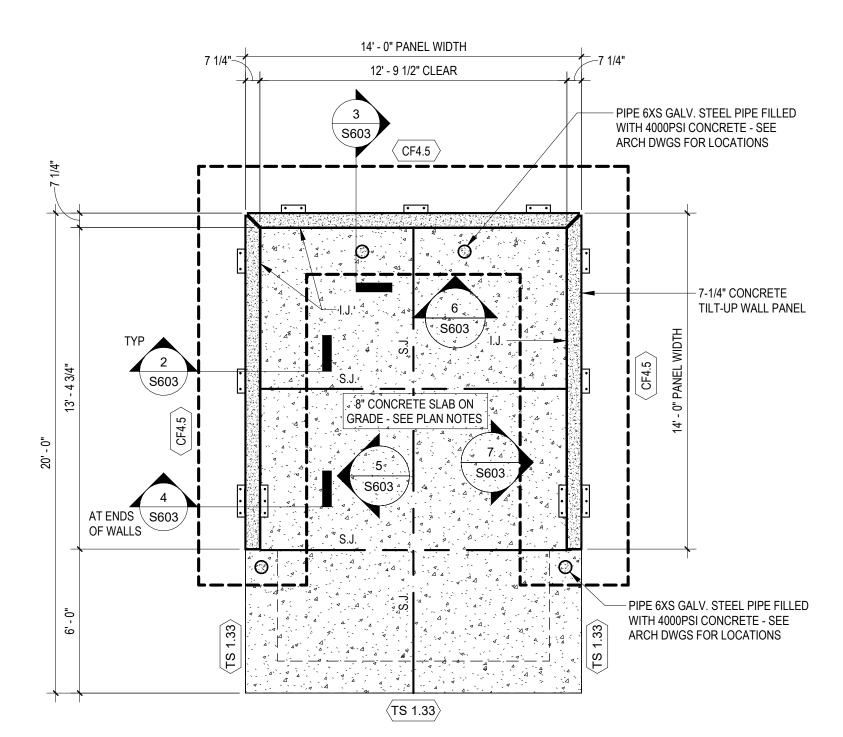
NO 74693

NO 74693

**
STATE OF
CONSIDER
CONSID

S602

Job Number:



FOUNDATION PLAN NOTES:

REINFORCEMENT.

- 1. REFERENCE THE STRUCTURAL GENERAL NOTES ON DRAWINGS S001 & S002. GENERAL NOTES INCLUDE CODES AND STANDARDS, DESIGN LOADS AND OTHER REQUIREMENTS.
- 2. PREPARE THE SLAB SUB-BASE AND COMPACT THE SOIL PER THE PROJECT GEOTECHNICAL REPORT, THE CIVIL DRAWINGS, AND THE STRUCTURAL GENERAL NOTES. IF ANY OF THESE DRAWINGS OR NOTES ARE IN CONFLICT, THE CONTRACTOR
- MUST ALERT THE ENGINEER FOR CLARIFICATION PRIOR TO START OF CONSTRUCTION. 3. T.O. FOOTING ELEVATION IS AT -1'-6" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION DETAIL SHEETS AND SCHEDULES FOR FOUNDATION SIZE AND
- 4. T.O. SLAB ELEVATION IS AT 0'-0" (U.N.O.) THIS IS A REFERENCE ELEVATION ONLY. SEE FOUNDATION AND SLAB ON GRADE DETAIL SHEETS.
- 5. SLAB ON GROUND IS 8" MINIMUM THICK 4,000 PSI CONCRETE SLAB U.N.O. REINFORCED WITH #4 AT 12" O.C. EACH WAY TOP & BOTTOM.
- 6. REFERENCE THE ARCHITECTURAL DRAWINGS FOR SLAB EDGES, FLOOR SLOPES, WALL OPENINGS, AND OTHER DIMENSIONS NOT GIVEN. CONTRACTOR MUST COORDINATE AND VERIFY ALL DIMENSIONS WITH PROJECT ARCHITECT PRIOR TO FABRICATION.

<u>LEGEND</u>

EL = #' - ##"
T.O. FTG INDICATES TOP OF CONCRETE FOOTING ELEVATION. INDICATES CONTINUOUS FOOTING TYPE. SEE FOUNDATION

SCHEDULE FOR SIZE AND REINFORCEMENT. INDICATES THICKENED SLAB EDGE TYPE. SEE FOUNDATION SCHEDULE FOR SIZE AND REINFORCEMENT.

ARCHITECTURE

135 W Central Blvd., Suite 400 Orlando, Florida 32801

TEL: 407.363.6136

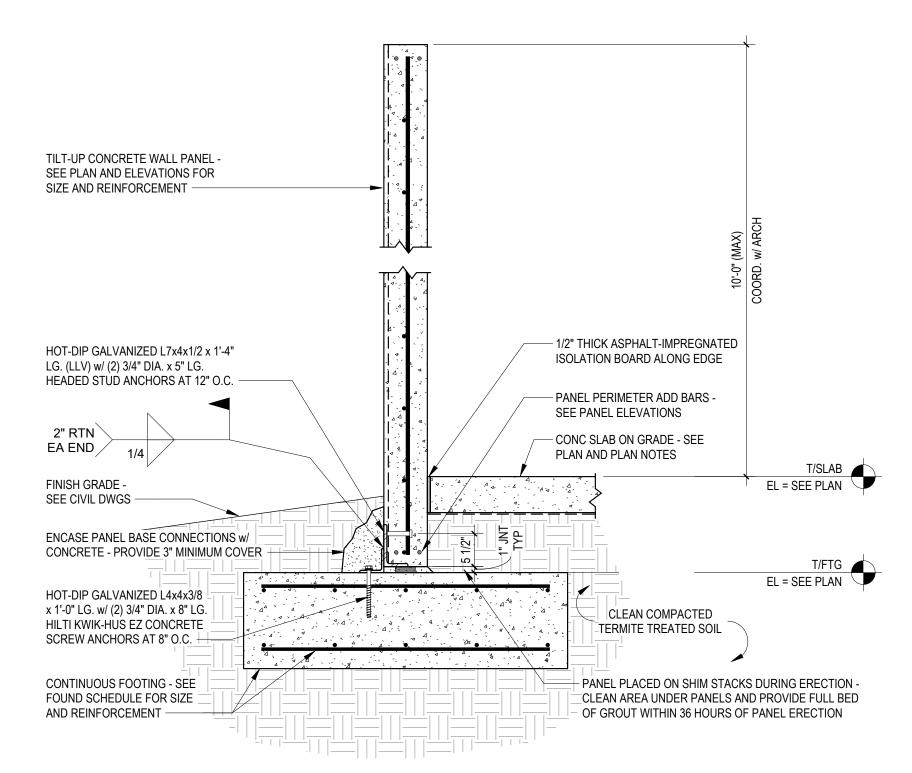
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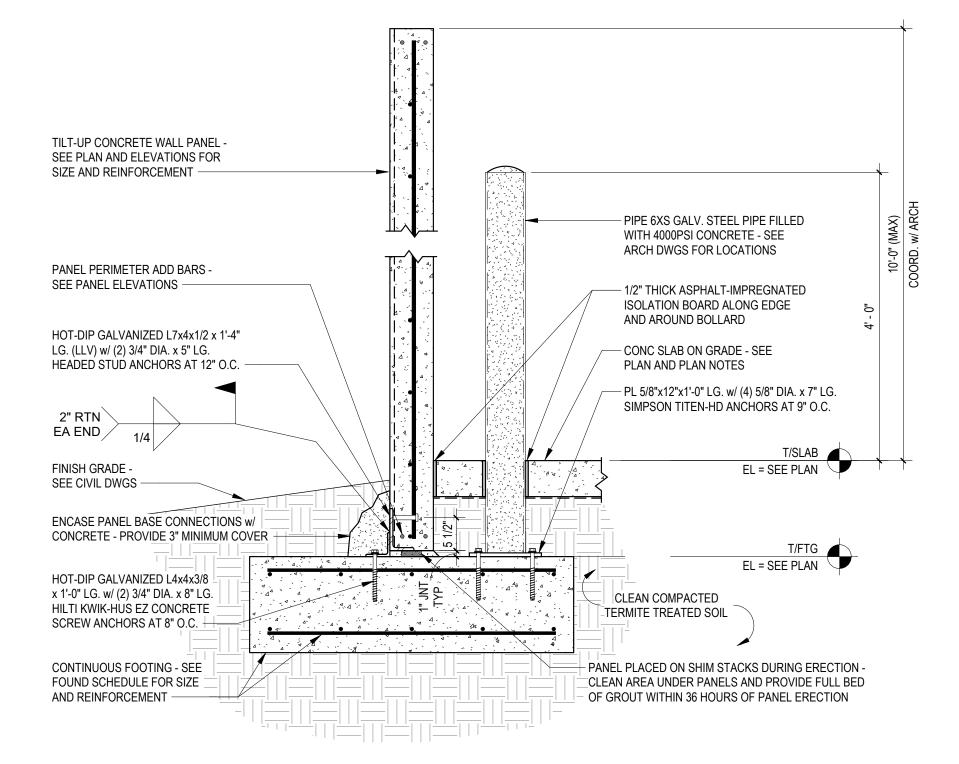
INDICATES SLAB SAWCUT JOINT PER DETAIL 8/S601 INDICATES ISOLATION JOINT WITH 1/2" THICK ASPHALT-IMPREGNATED BOARD PER TYPICAL DETAIL 12/S601

		<u>C(</u>	ONCRE	TE FOOTING SCHEDULE		
TYPE		SIZE		REINFORCING	REMARKS	
ITE	WIDTH	LENGTH	DEPTH	KLINI OKCING	INLIVIANNO	
(CF 4.5)	4' - 6"	CONT.	1' - 4"	LONG. (5) #5 x CONT. TOP & BOTTOM	CONTINUOUS FOOTING	
GF 4.5	4-0	CONT.	1 - 4	TRANS. #4 x 4'-0" LG. AT 12" O.C. TOP & BOTTOM	CONTINUOUSTOOTING	
(TS 1.33)	1' - 4"	CONT.	1' - 0"	LONG. (2) #5 x CONT. TOP LONG. (2) #5 x CONT. BOTTOM	THICKENED SLAB EDGE	
15 1.33	1 - 4	CONT.	1-0	ALT. TRANS. #4 x 4'-0" & 5'-6" LG. w/ ACI 90° HOOK TOP #4 x 1'-0" LG. SKEWED SUPPORT BAR AT 36" O.C. BOT	THICKEINED SLAB EDGE	

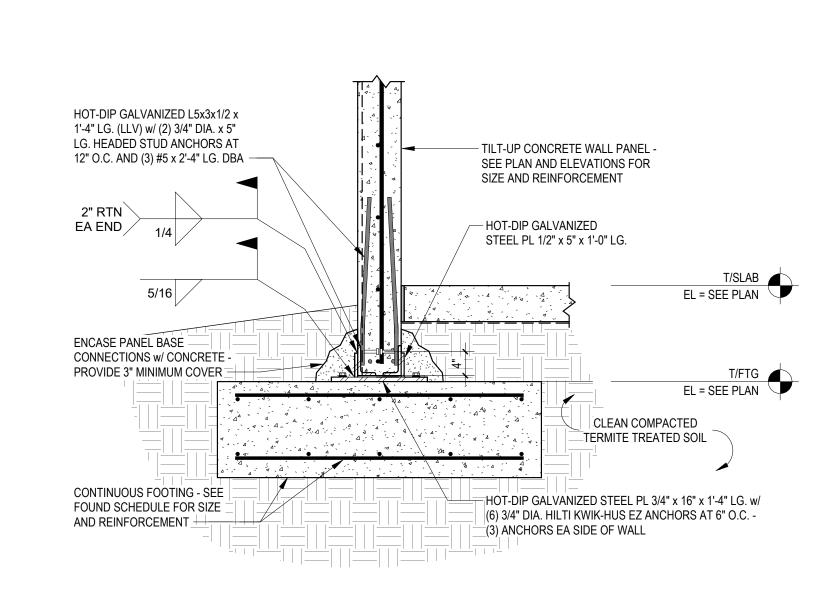
FOUNDATION PLAN - AT DUMPSTER AREA



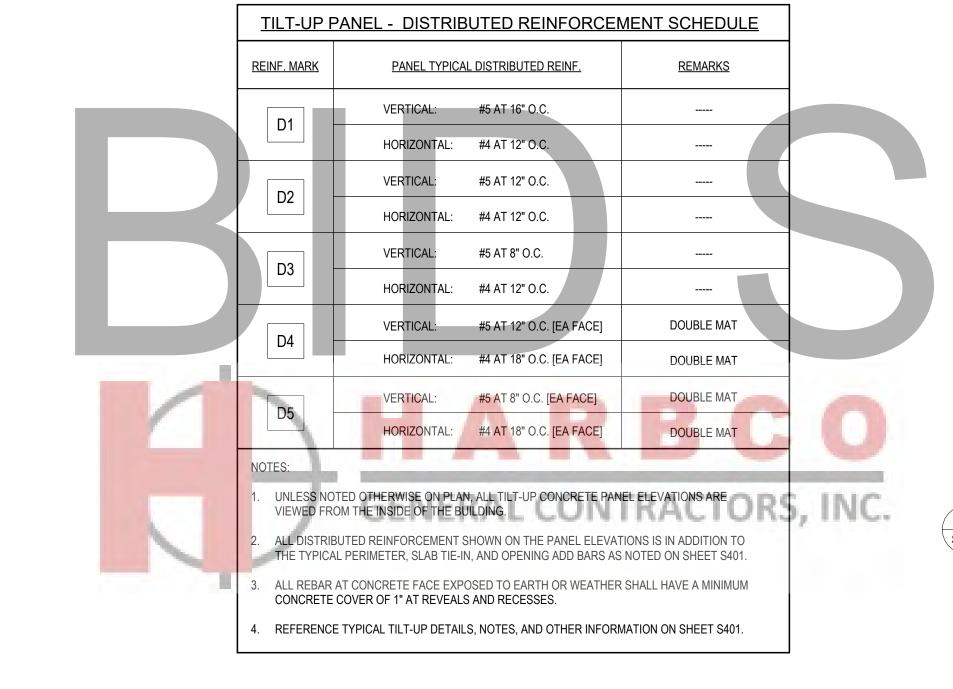


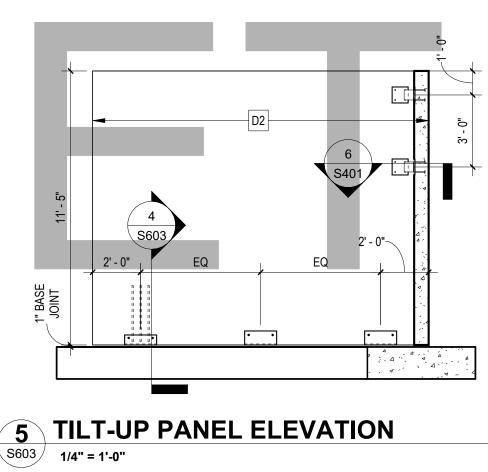


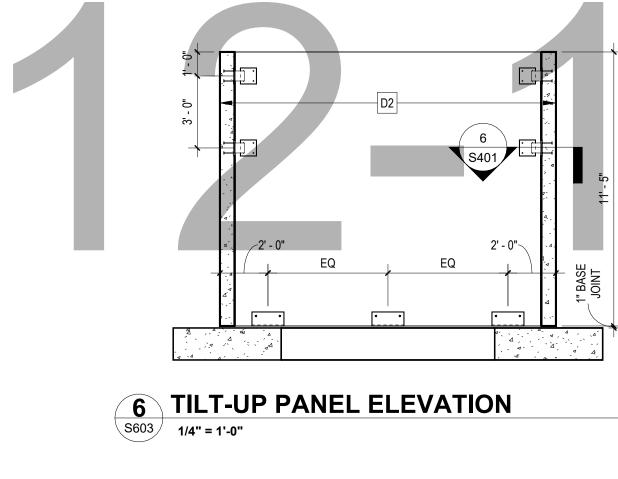
3 TYPICAL TILT-UP SCREEN WALL AT DUMPSTER WITH BOLLARD

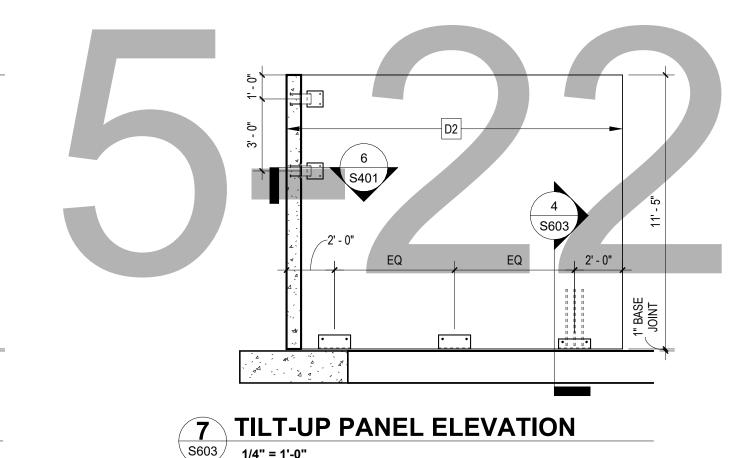


4 PANEL TO FOOTING CONNECTION AT DISCONTINUOUS ENDS





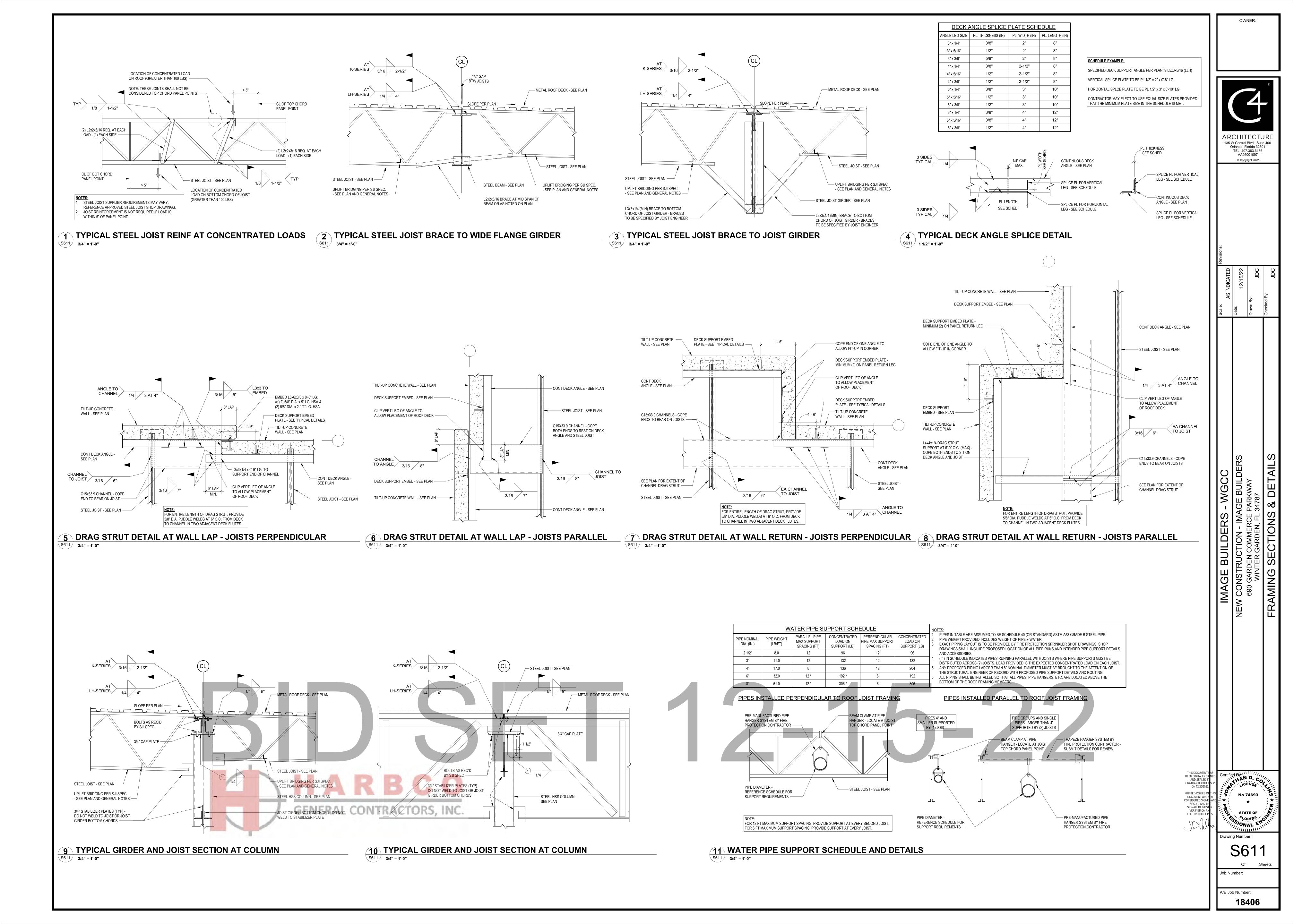


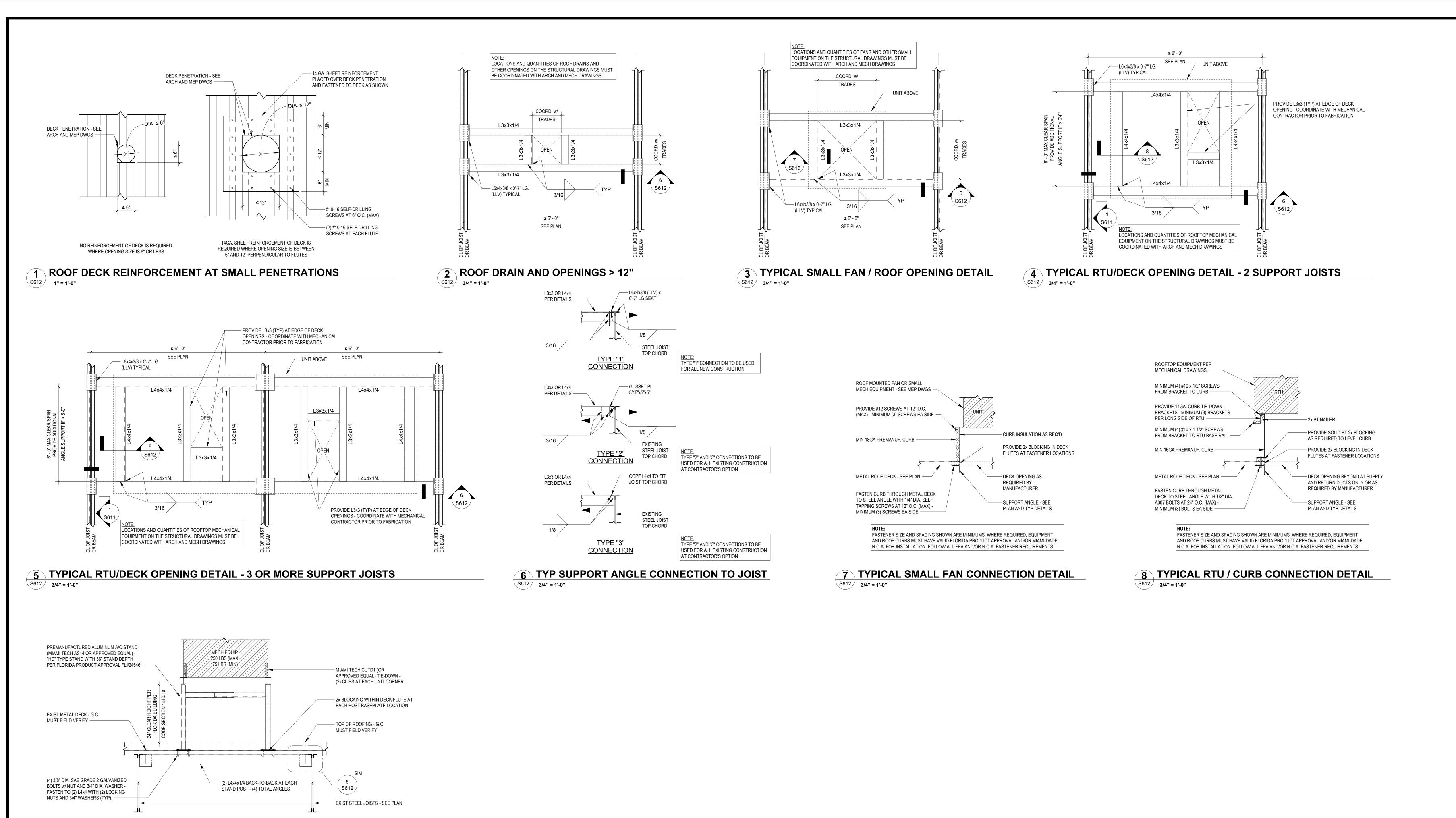


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9 TYPICAL PRE-ENGINEERED A/C STAND SUPPORT DETAIL

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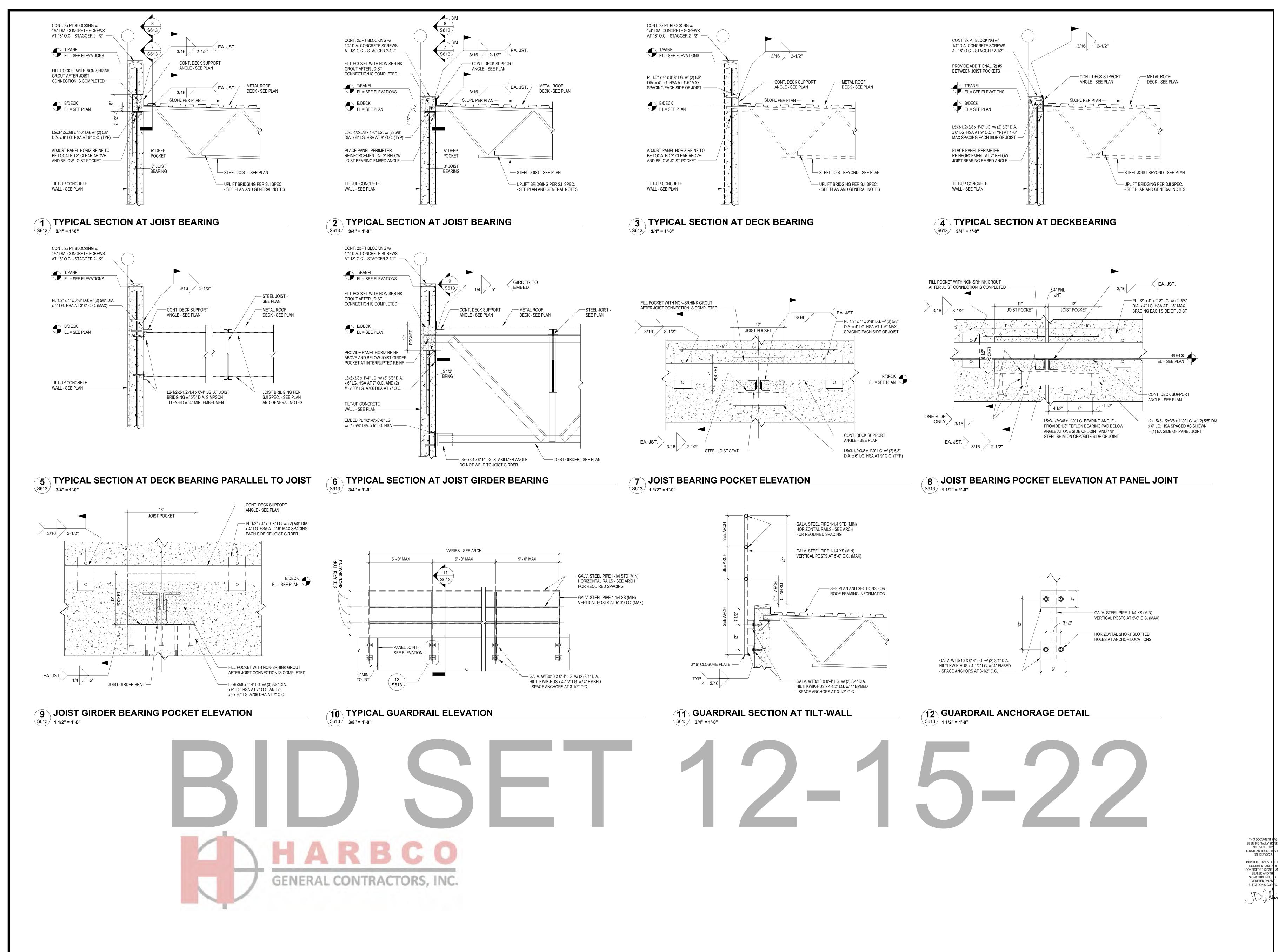
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ARCHITECTURE 135 W Central Blvd., Suite 400 Orlando, Florida 32801

TEL: 407.363.6136

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ARCHITECTURE

135 W Central Blvd., Suite 400
Orlando, Florida 32801
TEL: 407.363.6136
AA26001097

RCHITECTURE
5 W Central Blvd., Suite 400
Orlando, Florida 32801
TEL: 407.363.6136
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